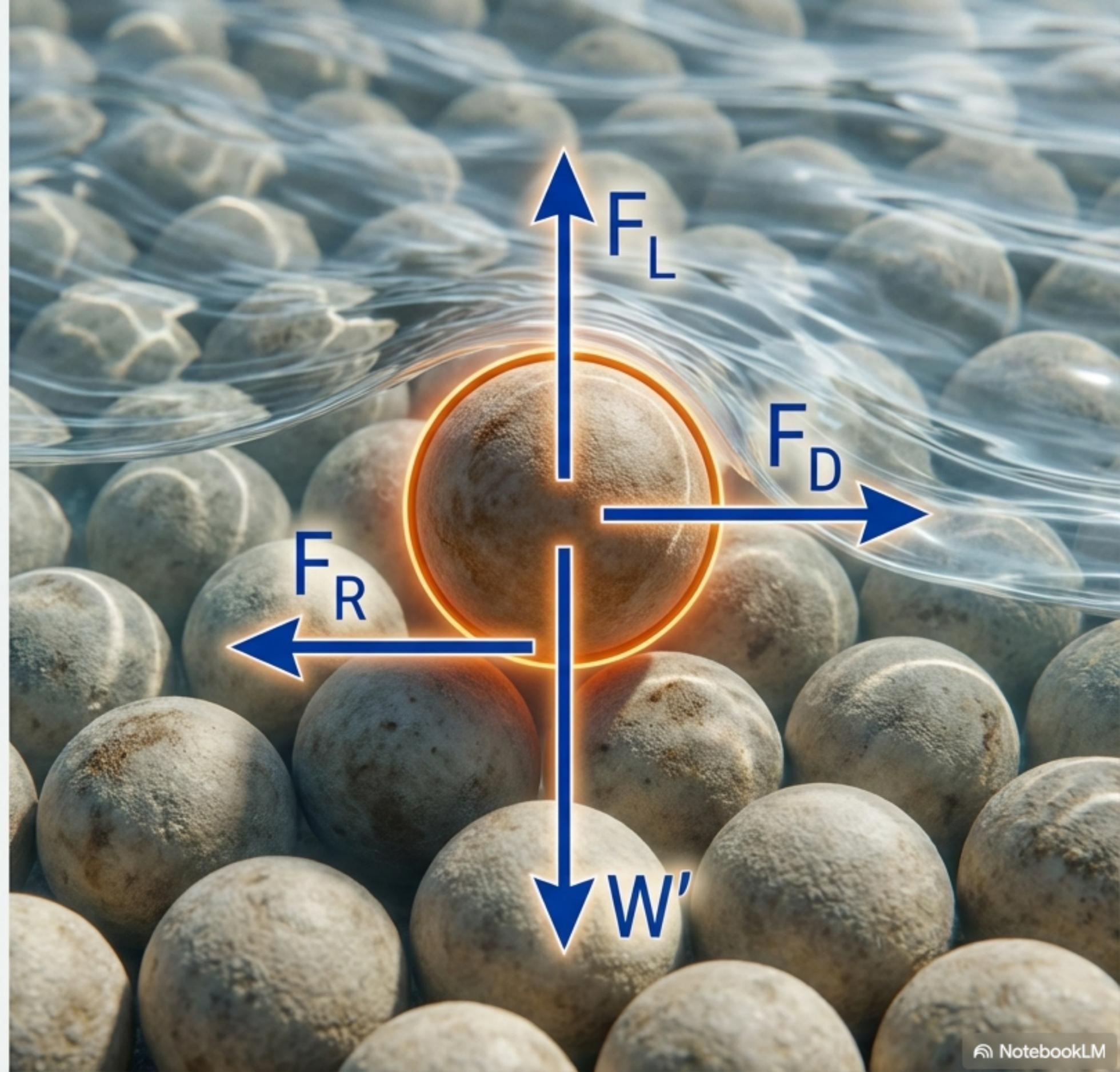


# Scour Criteria & Stable Channel Design

From Particle Physics to Hydraulic Geometry

- **The Goal:** To derive the geometry of a stable channel where the sediment is at the threshold of motion but does not scour.
- **The Logic Chain:** Particle Mechanics → Dimensionless Threshold → Stable Channel Geometry.
- **The Scope:** Non-cohesive sediment, uniform particle diameter ( $d$ ), clear-water threshold, steady uniform flow.
- **Key Definition:** Incipient motion occurs when applied boundary shear stress equals critical shear stress.

$$\tau_0 = \tau_c$$



# The Micro Scale: Force Balance on a Single Grain

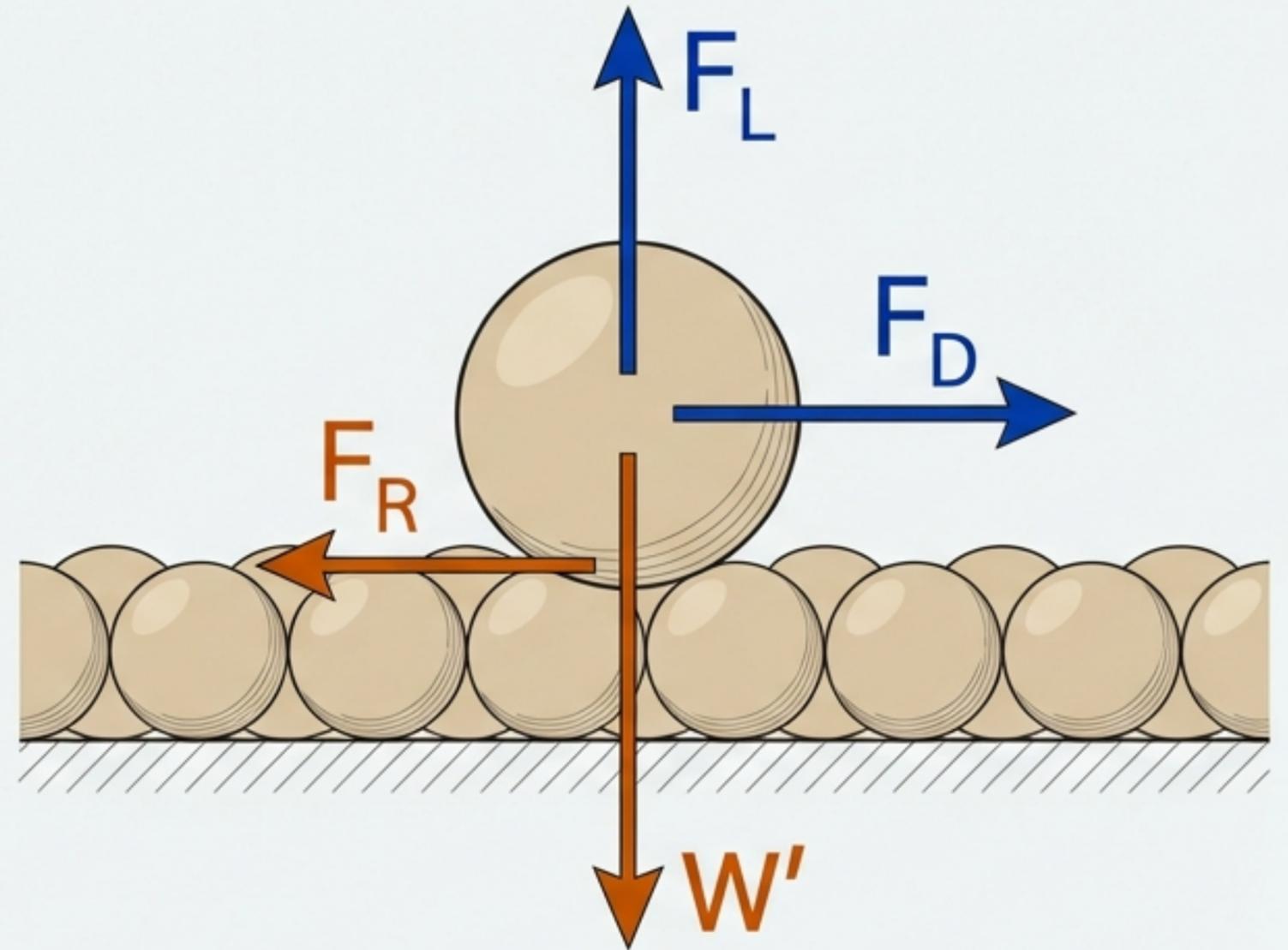
The stability of a channel begins with a single grain.

Hydrodynamic Forces:

The flow exerts Drag ( $F_D$ ) pushing the particle downstream and Lift ( $F_L$ ) attempting to raise it from the bed.

Resisting Forces:

The particle is anchored by its Submerged Weight ( $W'$ ) and the resulting Frictional Resistance ( $F_R$ ).



Threshold Condition:  $F_D = \mu W' - F_L$

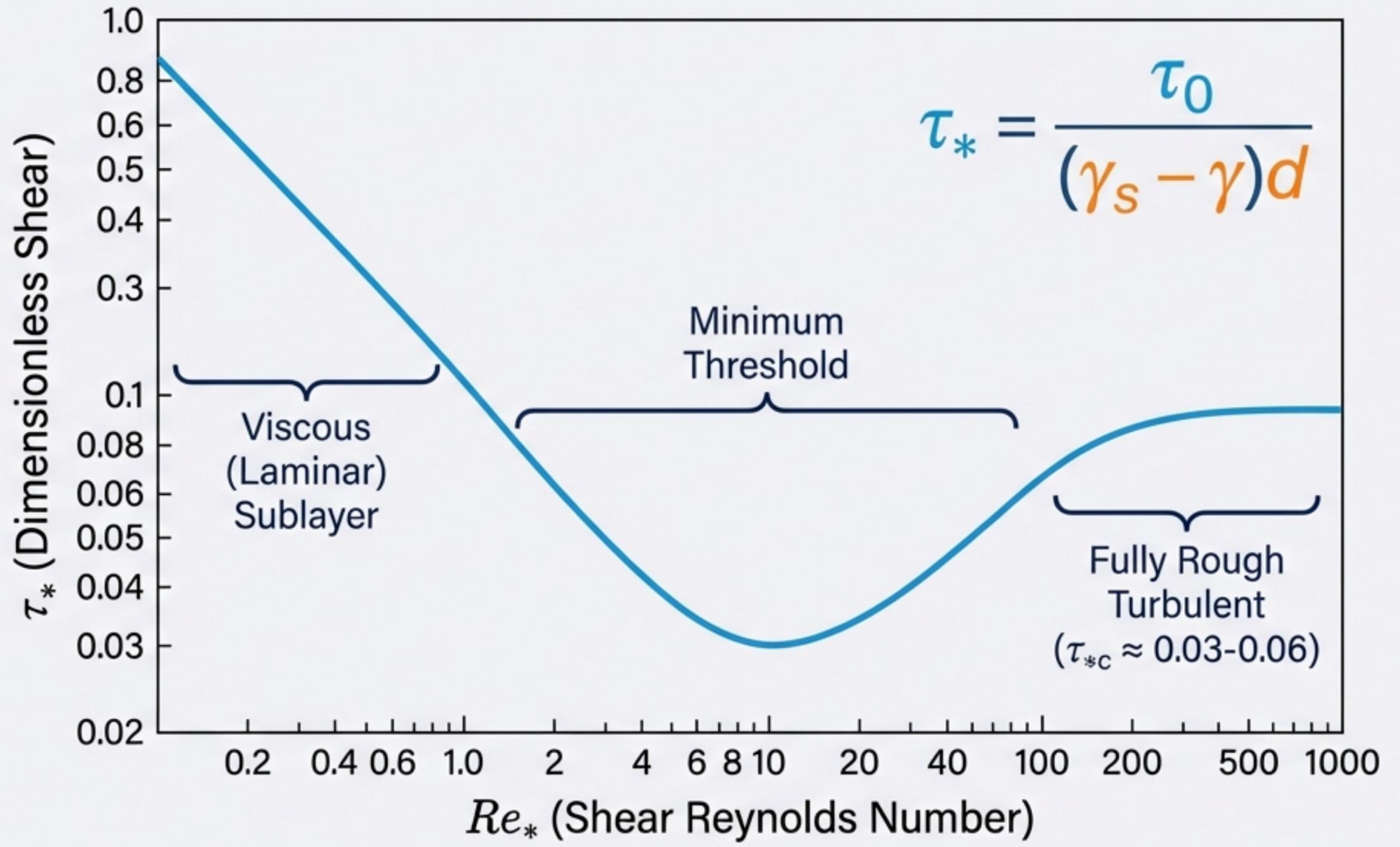
Friction Coefficient:  $\mu = \tan(\phi)$

Typical  $\phi$  values: Sand (28–34°), Gravel (up to 40°)

# The Gold Standard: The Shields Parameter

**Albert F. Shields**  
(1936) normalized  
particle stability into  
a dimensionless  
threshold.

The curve  
represents the  
boundary between  
"Stable" (below) and  
"Motion" (above).



# The Mechanism: The Coleman–Ikeda–Iwagaki (C–I–I) Model

## The Problem

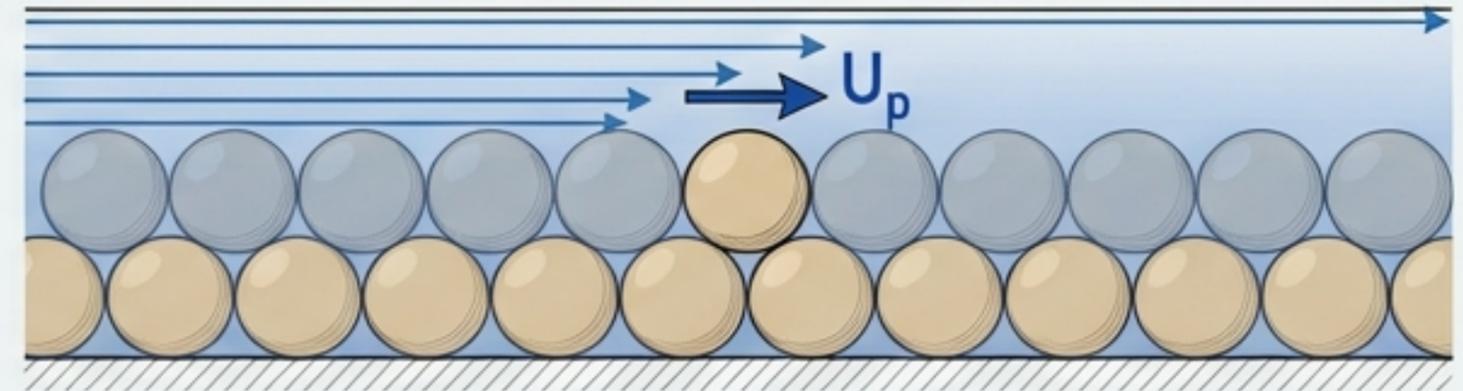
Shields is empirical. To understand the physics, we must look at the specific velocity acting on the particle ( $U_p$ ), not the depth-averaged velocity.

## The Hiding Factor:

- Small  $Re_*^*$ : The particle hides in the viscous sublayer. The velocity it feels is small, making it harder to move (Curve rises).
- Large  $Re_*^*$ : The particle protrudes into the turbulent layer. It is fully exposed to flow (Curve levels off).

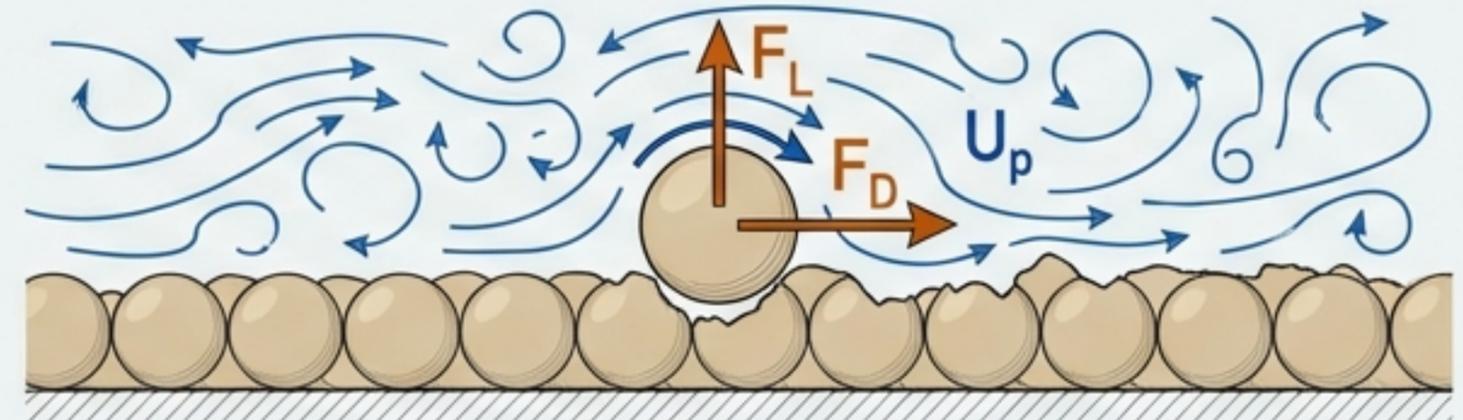
## Visual Definition

Viscous Limit ( $Re_* \lesssim 5$ )



$$f(Re_*) = \frac{Re_*}{2} \text{ (Linear)}$$

Rough Limit ( $Re_* \gtrsim 70$ )



$$f(Re_*) \approx 6.7 \text{ (Log-law)}$$

# Mathematical Derivation of the Critical Threshold

**Step 1: Force Balance:**

$$\frac{1}{2}\rho C_D A U_p^2 = \frac{\mu W'}{1 + \alpha_L \mu}$$

(Driving Drag = Resisting Friction (adjusted for Lift))

**Step 2: Solving for Particle Velocity ( $U_p$ ):**

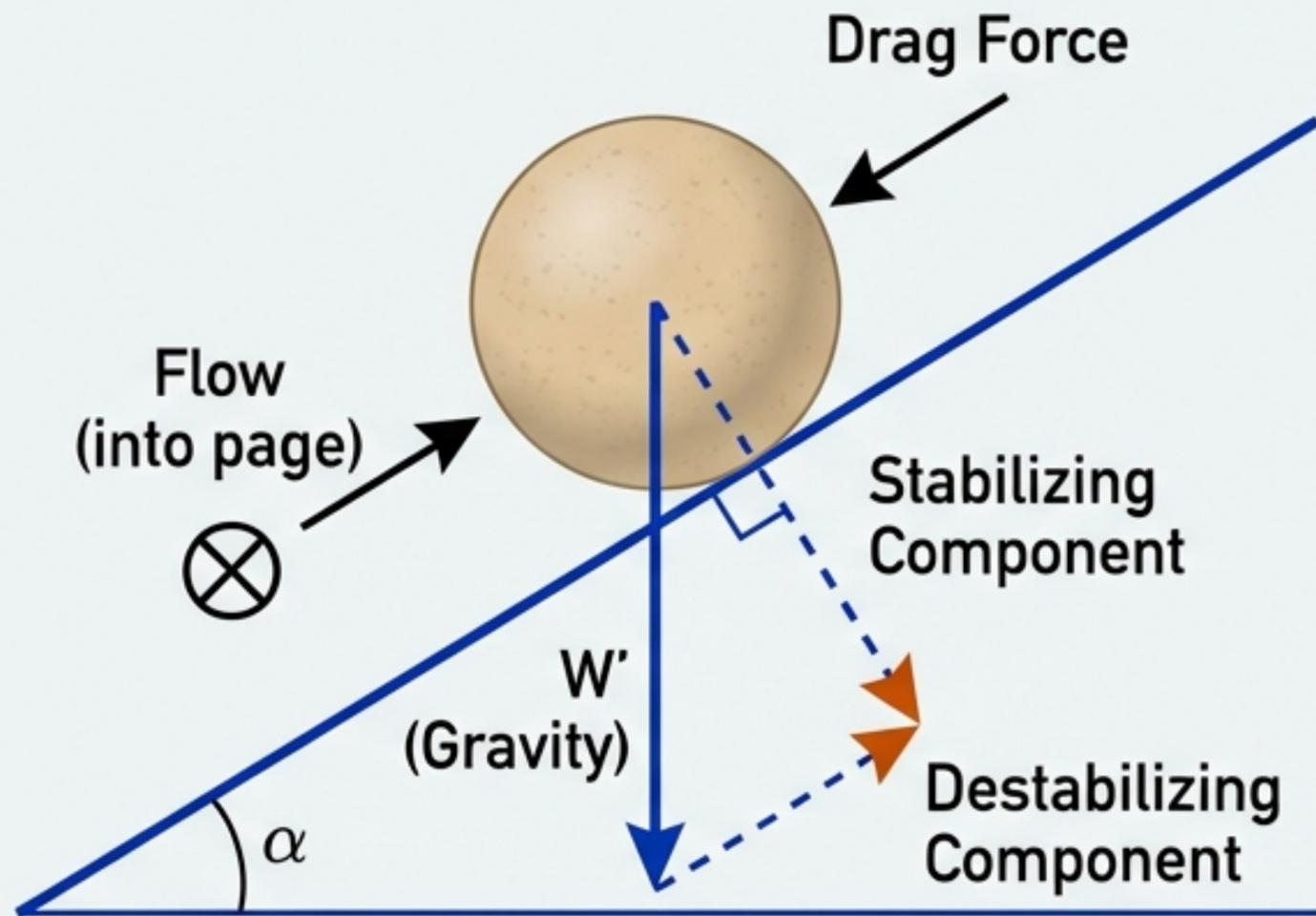
$$U_p^2 = \frac{4\mu}{3C_D(1 + \alpha_L \mu)} Rgd$$

**Step 3: The C-I-I Relationship (Substituting  $U_p = f u_*$ ):**

$$\tau_{*c} = \frac{u_*^2}{Rgd} = \frac{4\mu}{3C_D(1 + \alpha_L \mu) f^2}$$

Since the velocity function ' $f$ ' changes with  $Re_*$ , the critical shear  $\tau_{*c}$  must change. This model mathematically predicts the dip and rise of the Shields curve.

# The Meso Scale: Stability on Side Slopes



On a flat bed, gravity ( $W'$ ) acts purely to resist motion.

On a bank with slope angle  $\alpha$ , gravity splits into two components:

1. Downslope component: Pulls the particle down the bank (Destabilizing).
2. Perpendicular component: Holds the particle against the bank (Stabilizing).

Result: A particle on a bank moves more easily than a particle on the bed.

Limit Condition: As  $\alpha \rightarrow \phi$  (Angle of Repose), the stable shear stress approaches zero.

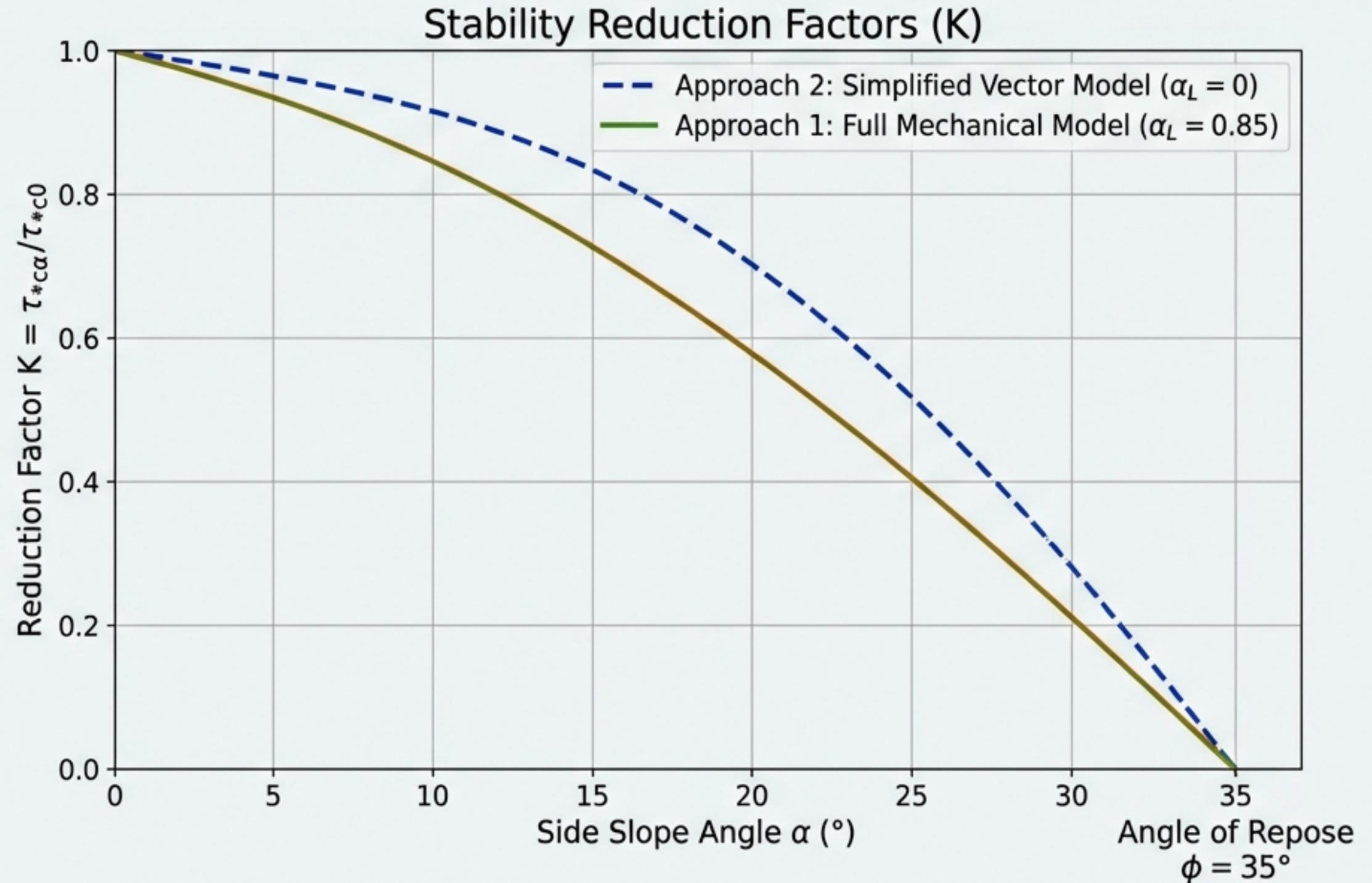
# The Reduction Factor (K)

We define  $K$  as the ratio of critical shear on a slope to critical shear on a flat bed:  $K < 1$ .

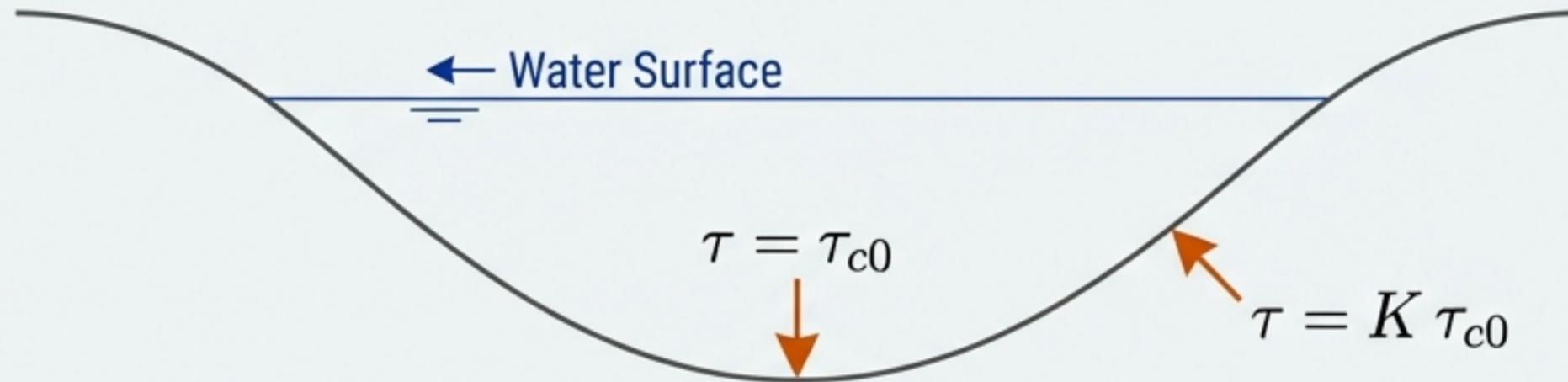
The Plot:

Approach 1 (Solid Line) includes the Lift Force. It shows a steeper reduction in stability, meaning the bank is more susceptible to scour than simplified models predict.

Design Implication: Using Approach 1 leads to safer, more conservative channel geometries.



# The Macro Scale: Simultaneous Incipient Motion



The 'Holy Grail' of stable channel design.

An ideal stable channel is not a box or a trapezoid. It is a shape where every point on the perimeter is exactly at the threshold of motion.

Efficiency: If shear < critical: Over-designed (too wide).

If shear > critical: Erosion (scour).

Result: Nature dictates a continuous curved bank profile.

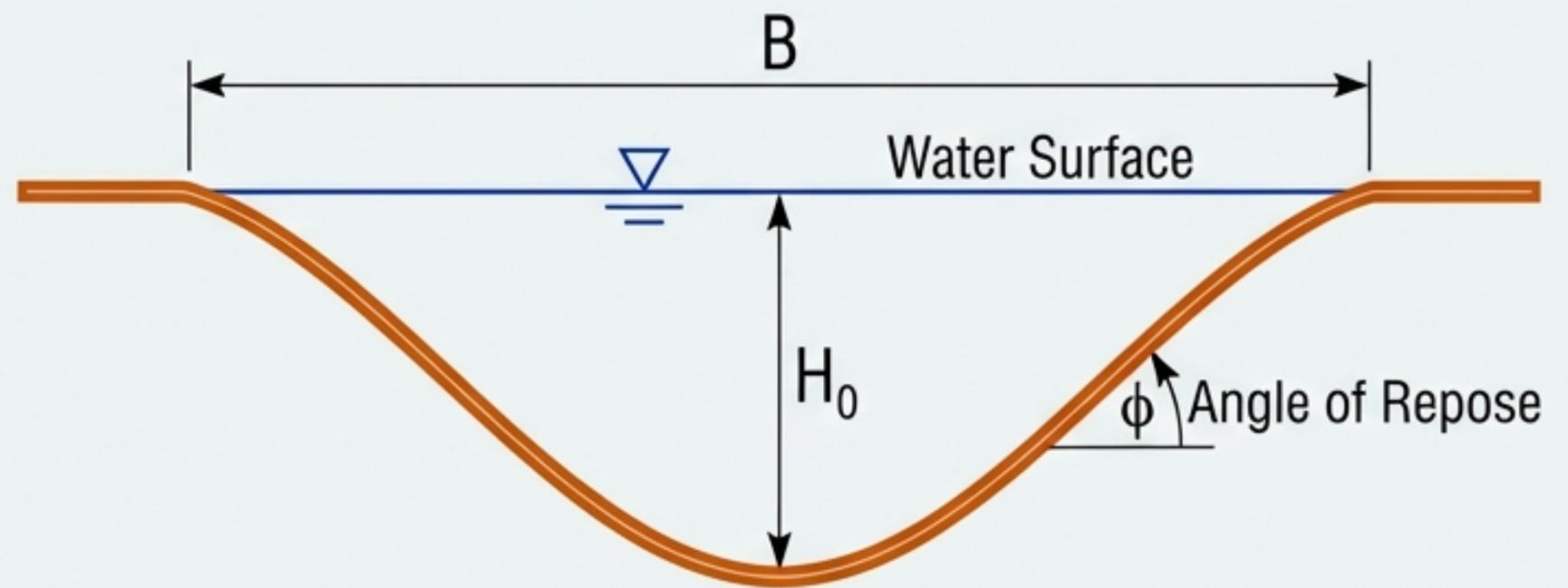
# The Glover-Florey-Lane (G-F-L) Profile

## Deriving the Shape:

By assuming local shear is proportional to depth ( $\tau \propto$  depth) and applying the K-factor reduction, we solve the differential equation for stability. The solution is a trigonometric cosine curve.

$$H(y) = H_0 \cos\left(\frac{y \tan(\phi)}{H_0}\right)$$

$$B = \frac{\pi H_0}{\tan(\phi)}$$



# The Shifted Cosine: Impact of Lift on Geometry

When Lift is included (Approach 1), the stable profile changes. The channel effectively **widens**. The cosine frequency scales to account for reduced effective friction on the bank.

**Practical Approximation:** For quick calculations, engineers use power-law approximations derived from the exact shape:

$$\frac{B}{H_0} \approx 3.14 \phi^{-1.038}$$

The Exact Equation (Approach 1):

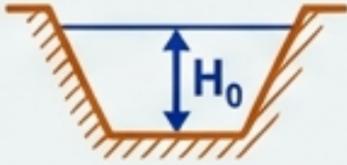
$$H(y) = H_0 [(1 - \alpha_L \mu)$$

$$\cos \left( \left( \frac{\mu}{1 - \alpha_L \mu} \sqrt{\frac{1 - \alpha_L \mu}{1 + \alpha_L \mu}} \frac{y}{H_0} \right) \right) - \alpha_L \mu ]$$

# Practical Design Workflow

- 

**1 Material Analysis**  
Determine particle size ( $d$ ) and Angle of Repose ( $\phi$ ).  
 $d, \phi$
- 

**2 Threshold Calculation**  
Calculate Critical Shields Stress ( $\tau_{*c}$ ) for the flat bed.  
 $\tau_{*c}$
- 

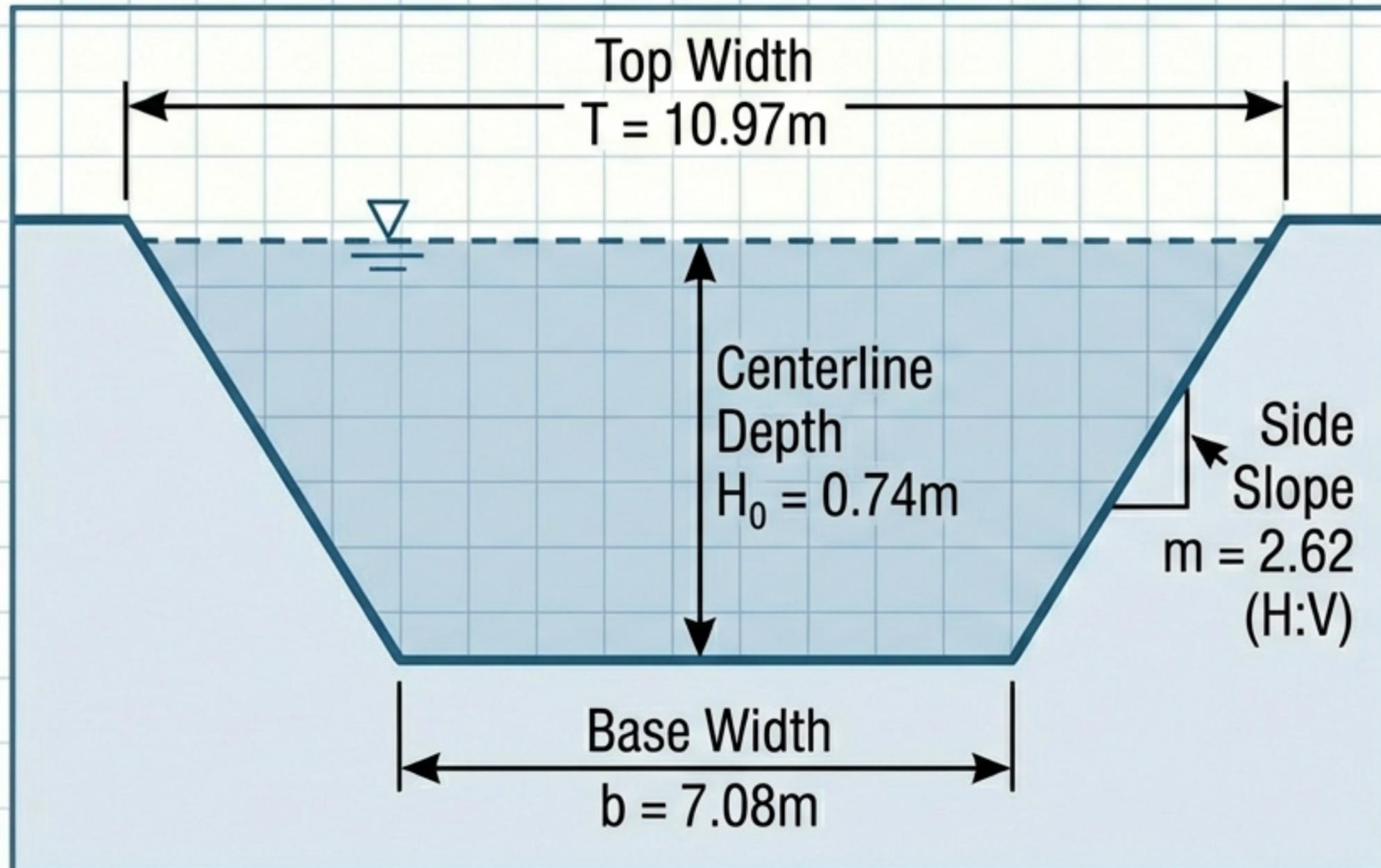
**3 Centerline Depth ( $H_0$ )**  
Solve using maximum allowable shear:  
$$H_0 = \frac{Rd\tau_{*c}}{S}$$
- 

**4 Side Geometry**  
Use the G-F-L relation ( $B/H_0$ ) to find the stable width or side slope.  
 $\frac{B}{H_0}$
- 

**5 Capacity Check**  
Use Manning's Equation to solve for the required bottom width ( $b$ ) to carry the design discharge ( $Q$ ).  
 $Q, b$

# Worked Example: Method A (Trapezoidal)

**Scenario Text:** Design a channel for  $Q = 5.0 \text{ m}^3/\text{s}$ ,  $d = 0.01\text{m}$ , Slope = 0.001.



## Calculation Summary

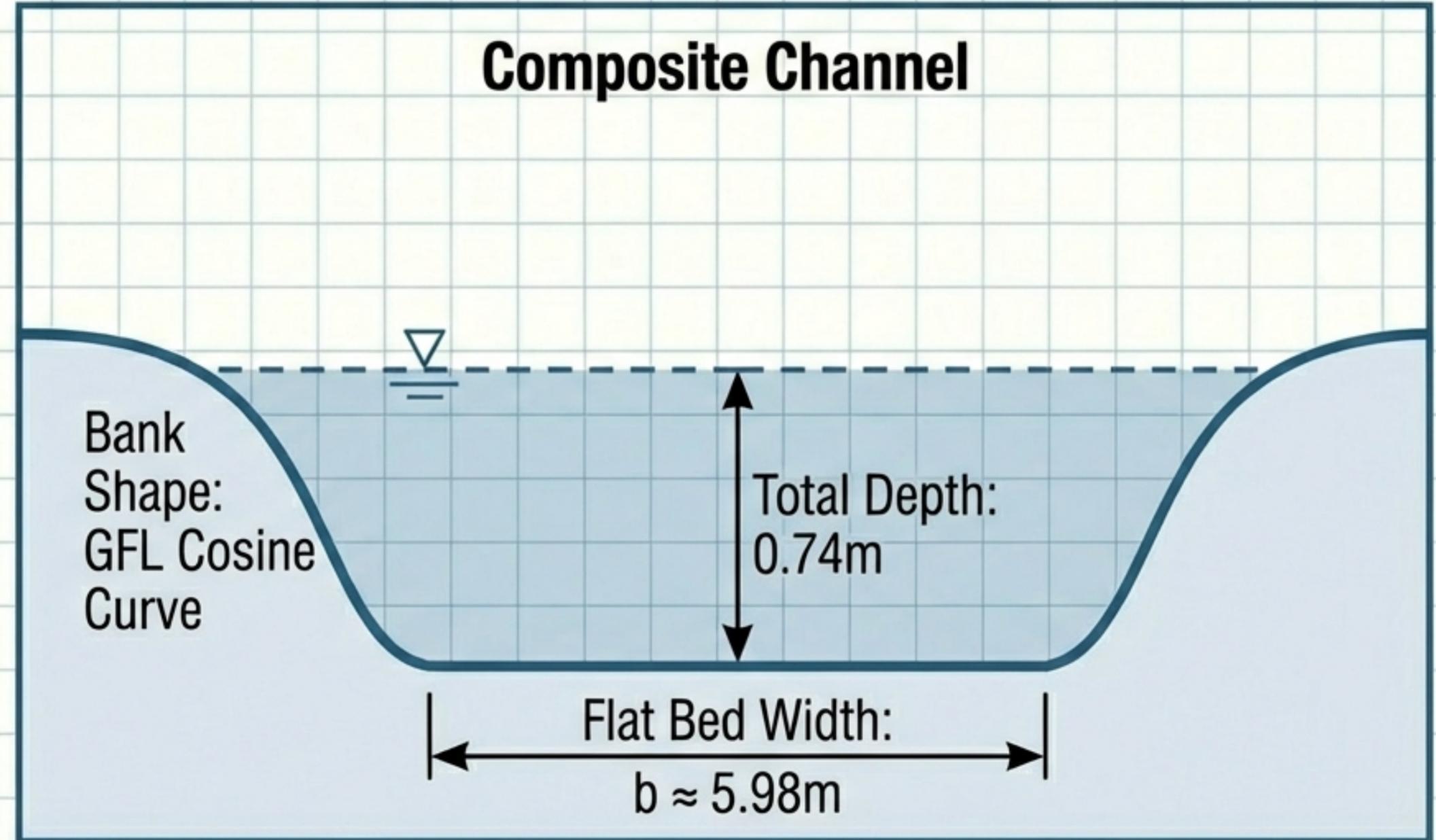
- **Step 1:**  $H_0$  derived from critical shear.
- **Step 2:** Side slope  $m$  derived from GFL approximation.
- **Step 3:** Base width  $b$  solved via Manning's Equation iteration.

# Worked Example: Method B (The Truly Ideal)

## Problem/Solution Text:

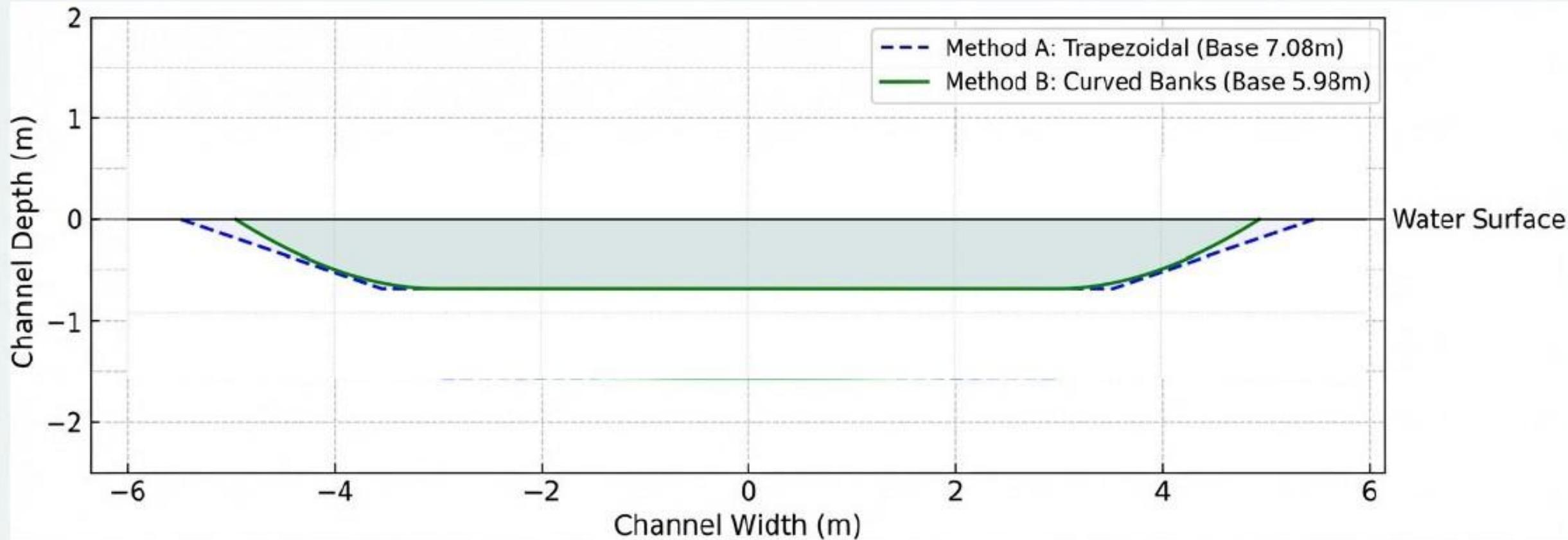
**Problem:** A pure cosine shape defined by GFL often lacks capacity. (Calculated  $Q \approx 1.21 \text{ m}^3/\text{s}$  vs Target  $5.0 \text{ m}^3/\text{s}$ ).

**Solution:** Composite Design. Insert a flat floor to carry the extra load while keeping curved curved banks for stability.



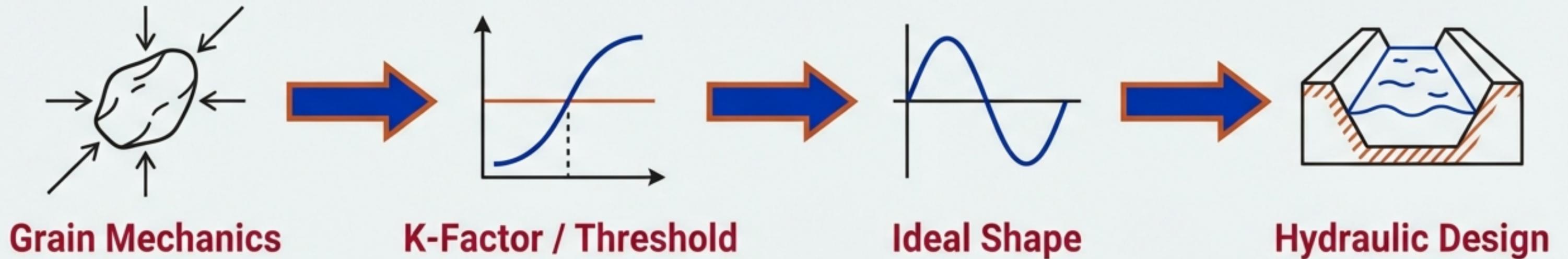
“Physics defines the curve; Engineering defines the width.”

# Design Comparison: Trapezoid vs. Ideal Curve



**Takeaway: Method B is more efficient (less excavation area for same Q), but Method A is easier to construct. The side slopes align closely, validating the trapezoidal approximation.**

# The Physical Foundation of Design



## Limitations & Context:

- **Probabilistic Nature:** Scour is a statistical probability, not instant.
- **Mixed Beds:** Hiding/exposure effects modify the simple Shields threshold.
- **Cohesion:** This workflow applies only to non-cohesive sediments (sand/gravel).

**Conclusion:** Understanding the physics of the single grain is the only way to reliably design the geometry of the channel.