

Transition Region

# Engineering Hydraulics: From Pipe to Outlet

Analysis of Pressurized Flow, Open  
Channel Systems, and Outlet Structures

1. Pressurized Systems
2. Gravity Systems
3. Outlet Control

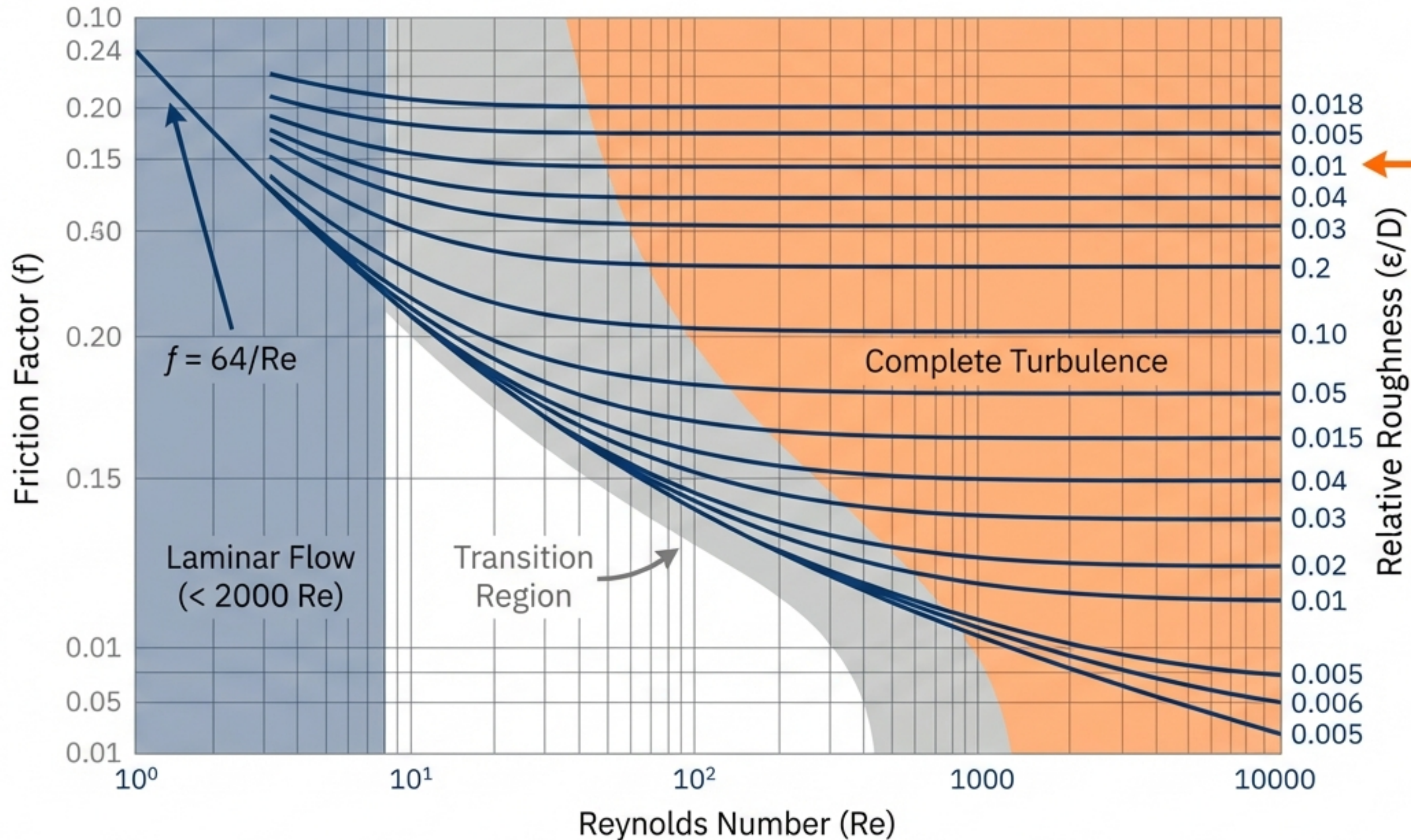
Laminar Flow

$$\frac{64}{Re}$$

Material	$\varepsilon$ (mm)
Concrete, coarse	0.25
Concrete, new smooth	0.025



# The Physics of Pipe Flow: Friction & Resistance

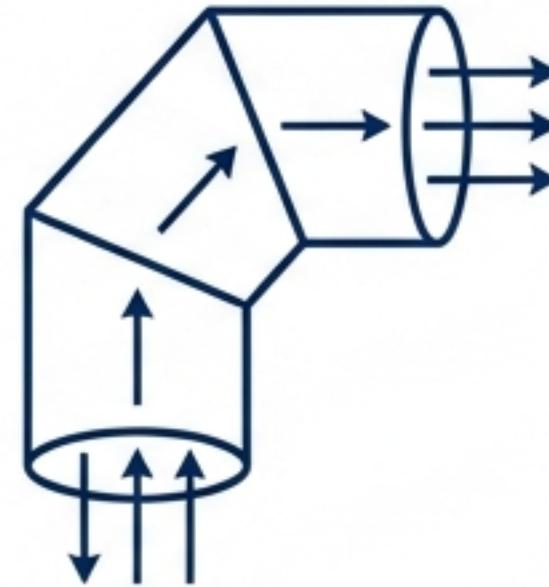
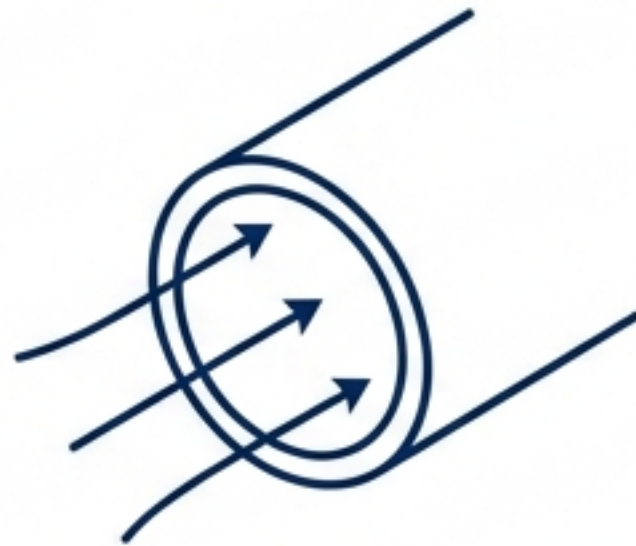
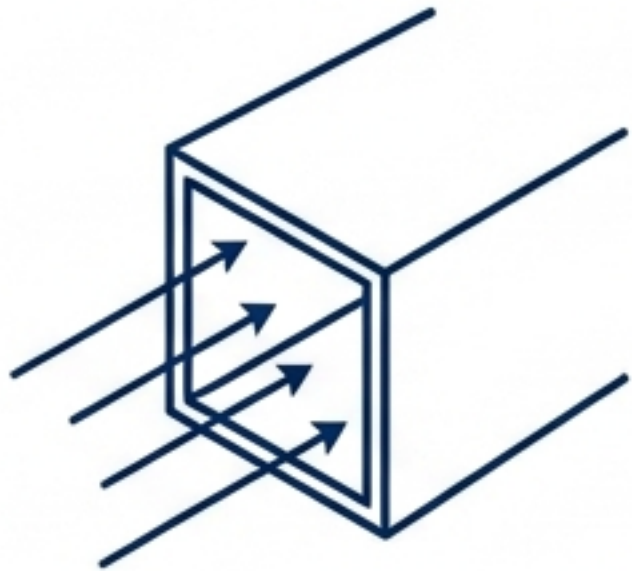


In turbulent flow, friction is driven by material roughness ( $\epsilon$ ), not fluid viscosity.

$$Re = \frac{\rho V d}{\mu}$$

# Local Losses: The Cost of Transitions

$$h_L = K_c \frac{V^2}{2g}$$



Square Edge Entrance:  
 $K = 0.50$   
Technical Grey IBM Plex Sans  
with IBM Plex Mono

Well-Rounded  
Entrance:  
 $K = 0.03$

90° Miter Bend:  
 $K = 1.1$

Globe Valve:  
 $K = 10.0$

**Key Insight:** Sharp transitions and complex valves act as significant energy sinks compared to smooth fittings.

# Adding Energy: Pump Flow Dynamics

Technical Grey  
Power Required (kW)

IBM Plex Mono  
Dynamic Head (meters) – The  
energy difference overcoming  
gravity and friction

$$P = (\gamma Q H) / \eta$$

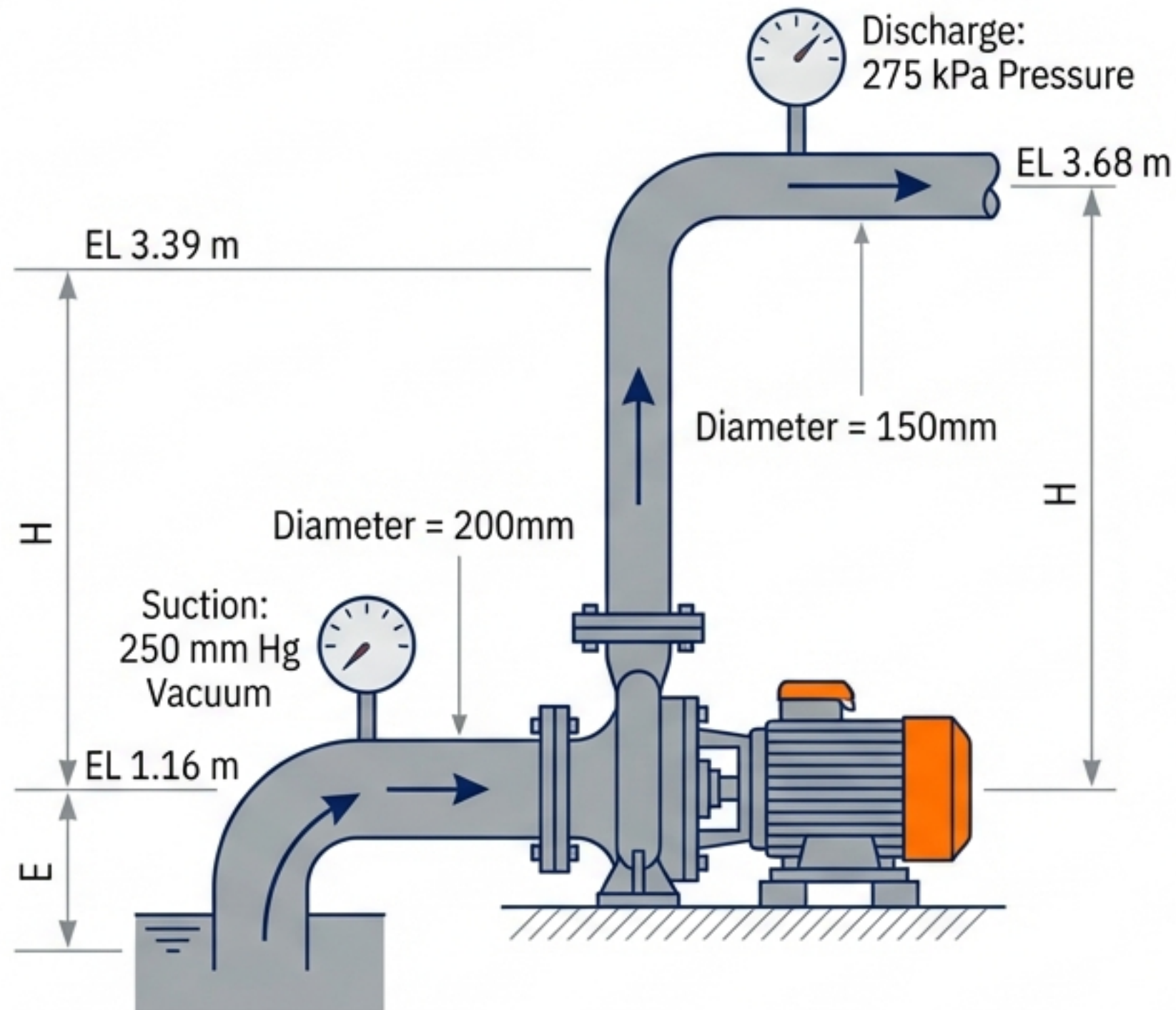
IBM Plex Mono  
Specific Weight of Water  
(9,810 N/m<sup>3</sup>)

IBM Plex Mono  
Flow Rate (m<sup>3</sup>/s)

IBM Plex Mono  
Efficiency (0.0 to 1.0)



# Case Study: Calculating Pump Requirements



## Problem Statement

**Goal:** Calculate the power (kW) required to deliver  $0.15 \text{ m}^3/\text{s}$  of water.

## Unit Conversion

1. Convert Gauge Pressures to Head (meters of water):

$$\text{Discharge Head} = \frac{275,000 \text{ Pa}}{9,800 \text{ N/m}^3} = 28.1 \text{ m}$$

$$\text{Suction Head} = -250 \text{ mm Hg} \times 13.57 \text{ (s.g.)} = -3.4 \text{ m}$$

# Case Study: Solution & Results

Step 1: Calculate Velocities (Continuity Eq)

$$V_{\text{suction}} = 4.77 \text{ m/s}$$

$$V_{\text{discharge}} = 8.48 \text{ m/s (Velocity increases as pipe narrows)}$$

Step 2: Apply Energy Equation IBM Plex Sans

$$\text{Total Head Increase } (E_p) = 37.0 \text{ Joules/Newton}$$

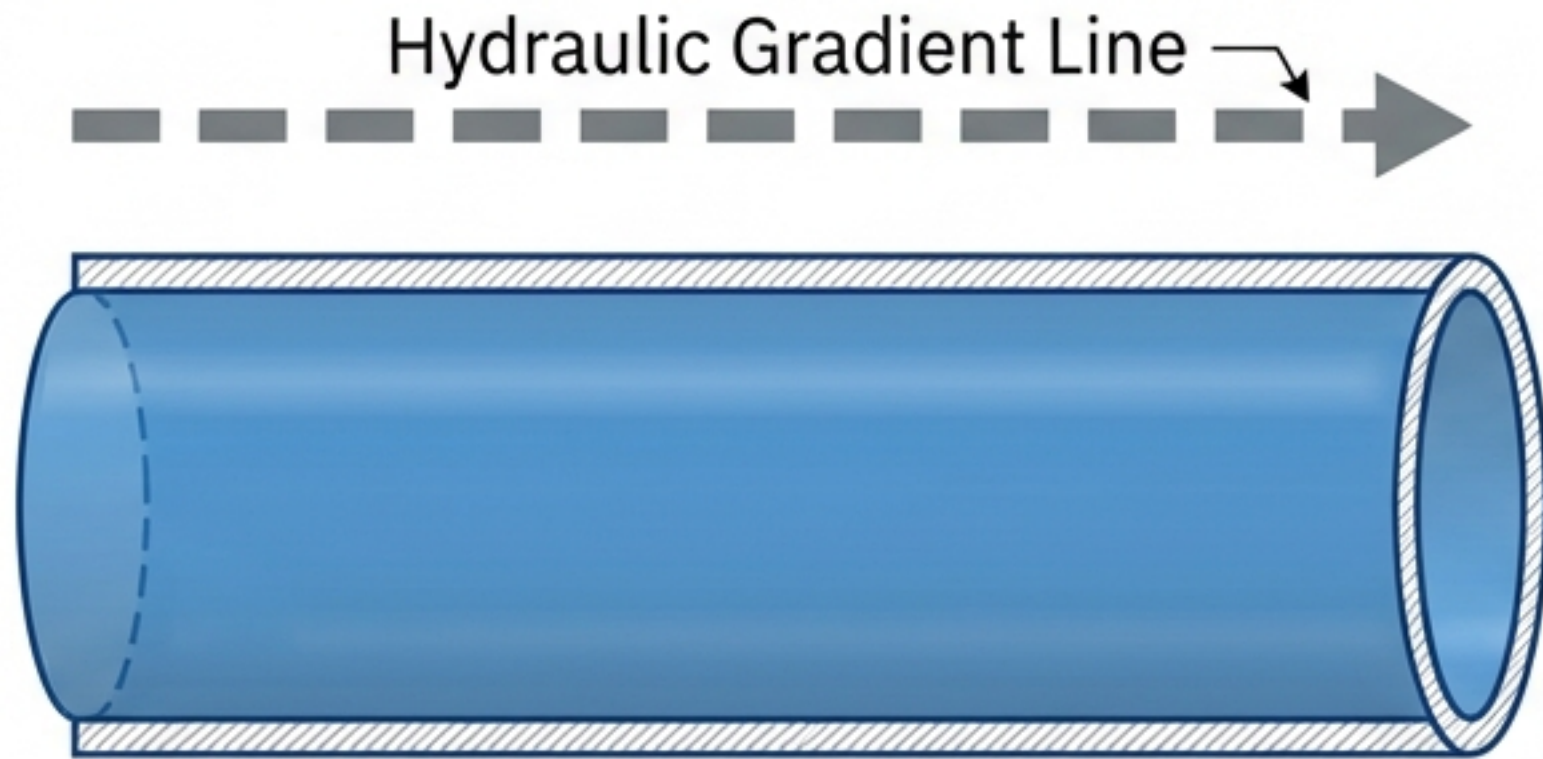
Step 3: Calculate Power IBM Plex Sans

$$P = (0.15 \times 9800 \times 37.0) / 1000 = 54.4 \text{ kW}$$

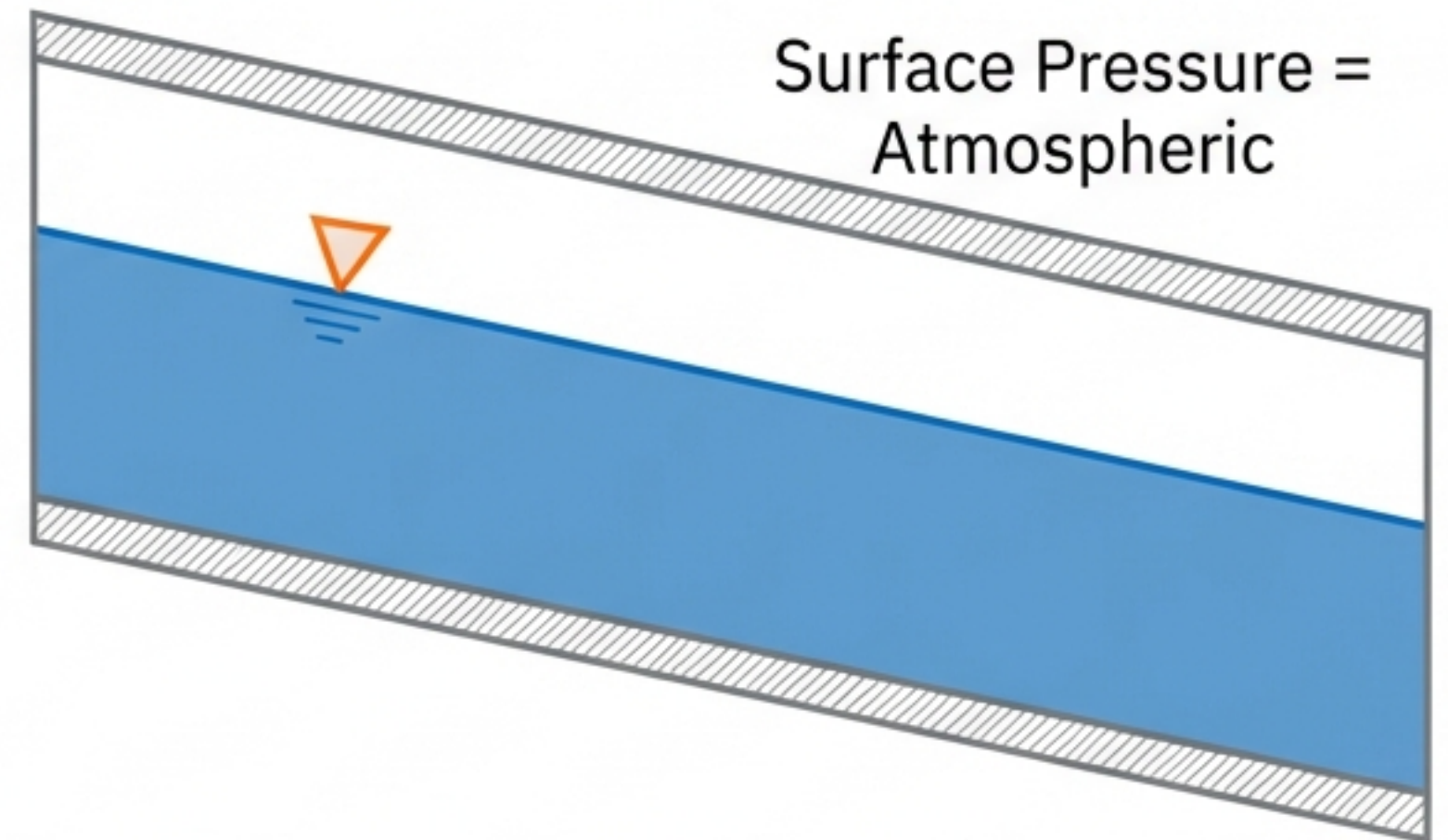
# Phase 2: Open Channel Flow

The Critical Difference: The Free Surface

## Pressurized Flow

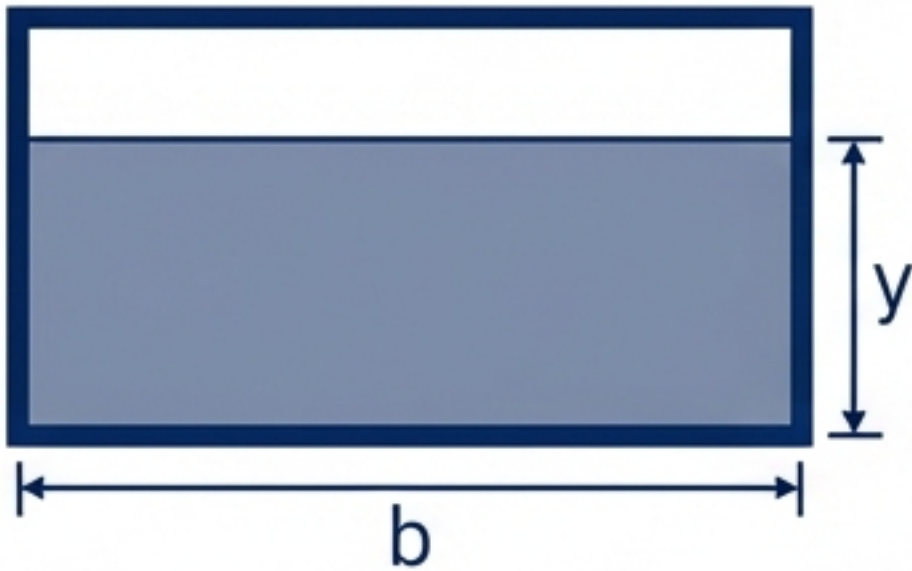


## Gravity Flow



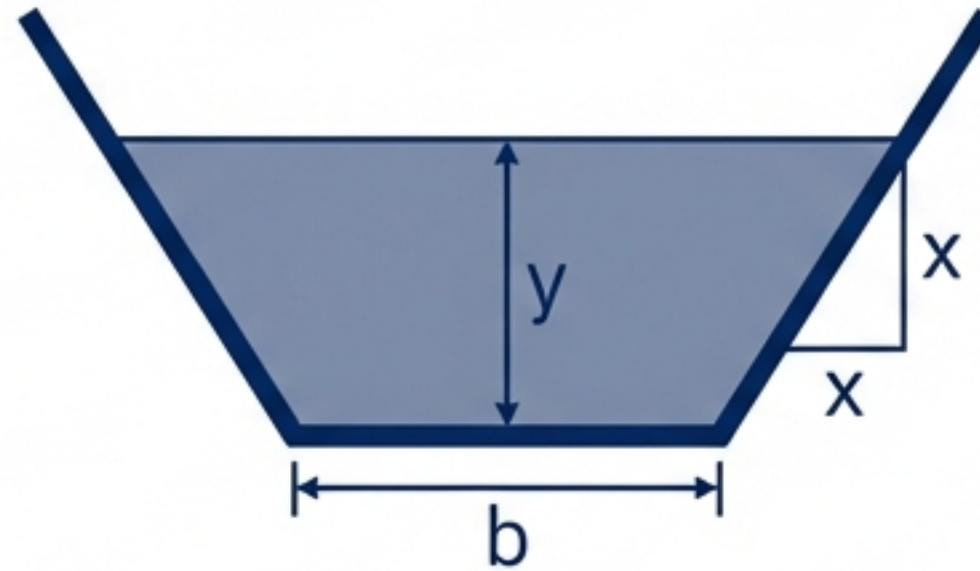
In open channels, water is driven by slope and gravity, not mechanical pressure.

# Channel Geometry & Hydraulic Radius



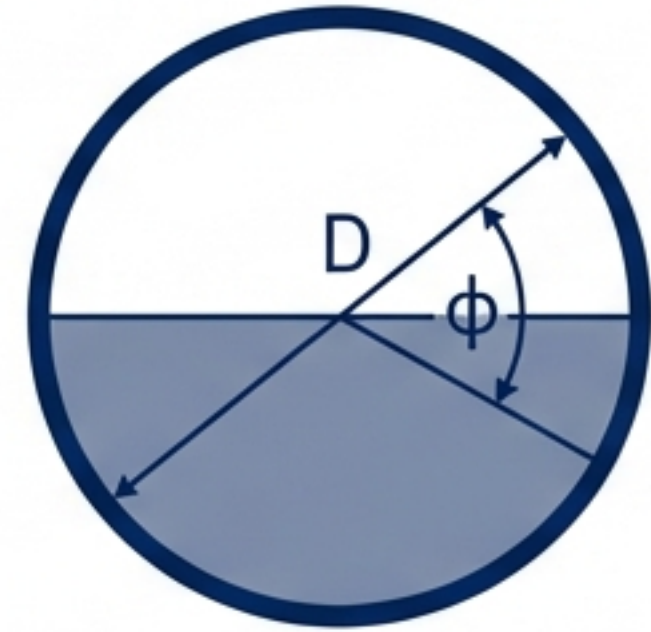
$$\text{Area (A)} = b \times y$$

$$\text{Wetted Perimeter (P)} = b + 2y$$



$$\text{Area (A)} = (b + xy)y$$

$$\text{Wetted Perimeter (P)} = b + 2y\sqrt{1+x^2}$$

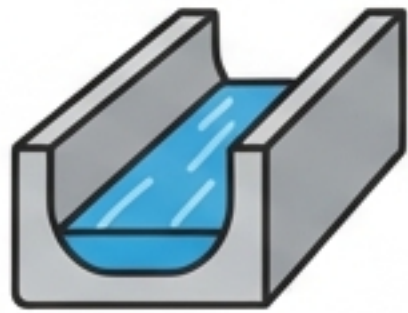


$$\text{Hydraulic Radius (R)} = \frac{A}{P}$$

Hydraulic Radius (R) is the ratio of the cross-sectional area to the length of the surface in contact with the water.

# The Manning Equation

$$V = (1/n) \times R^{2/3} \times S^{1/2}$$



Finished Concrete  
 $n = 0.013$   
(Smooth, Fast)



Clean Stream  
 $n = 0.030$

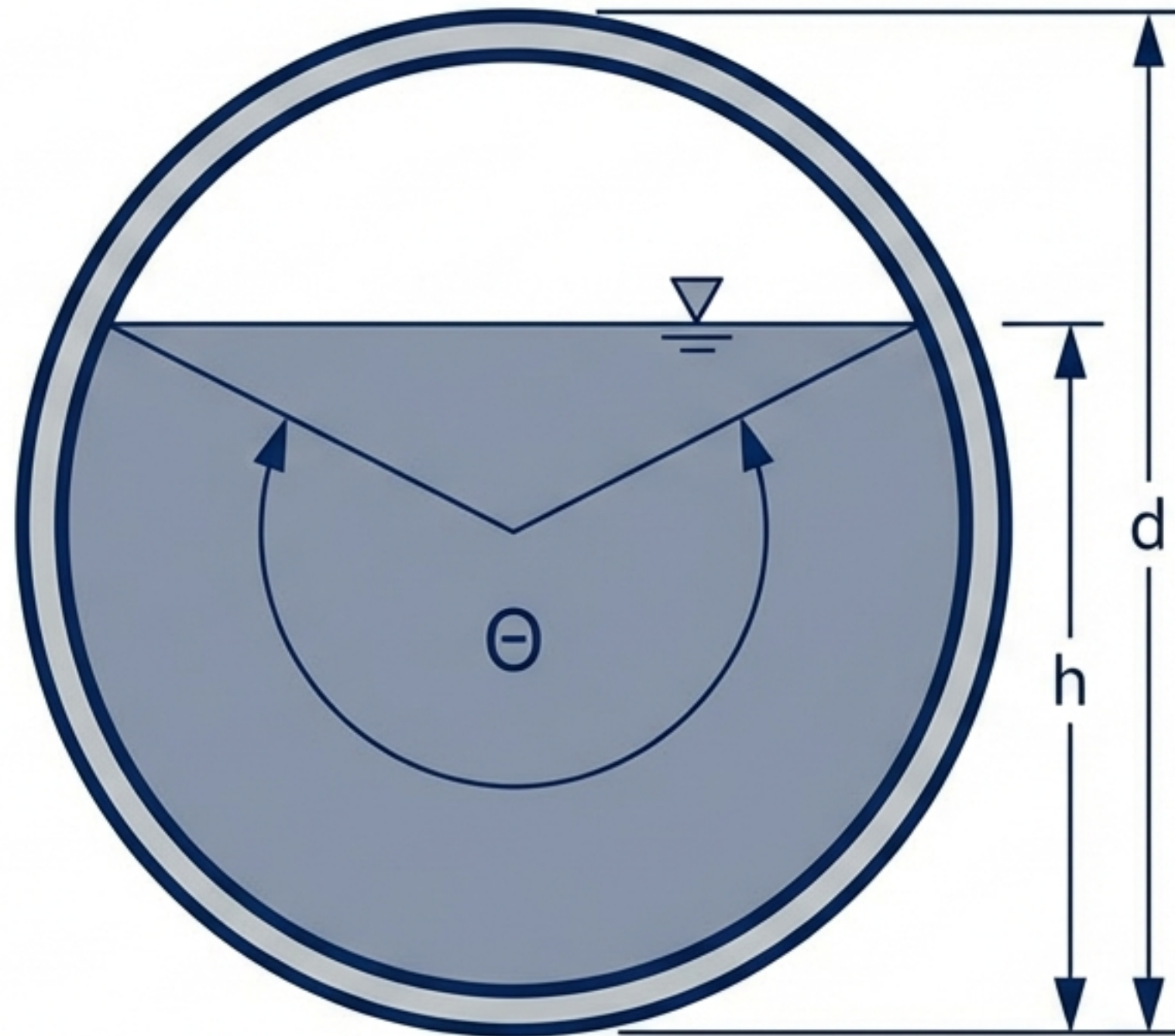


Dense Brush/Weeds  
 $n = 0.080$   
(Rough, Slow)

Spectrum of Roughness

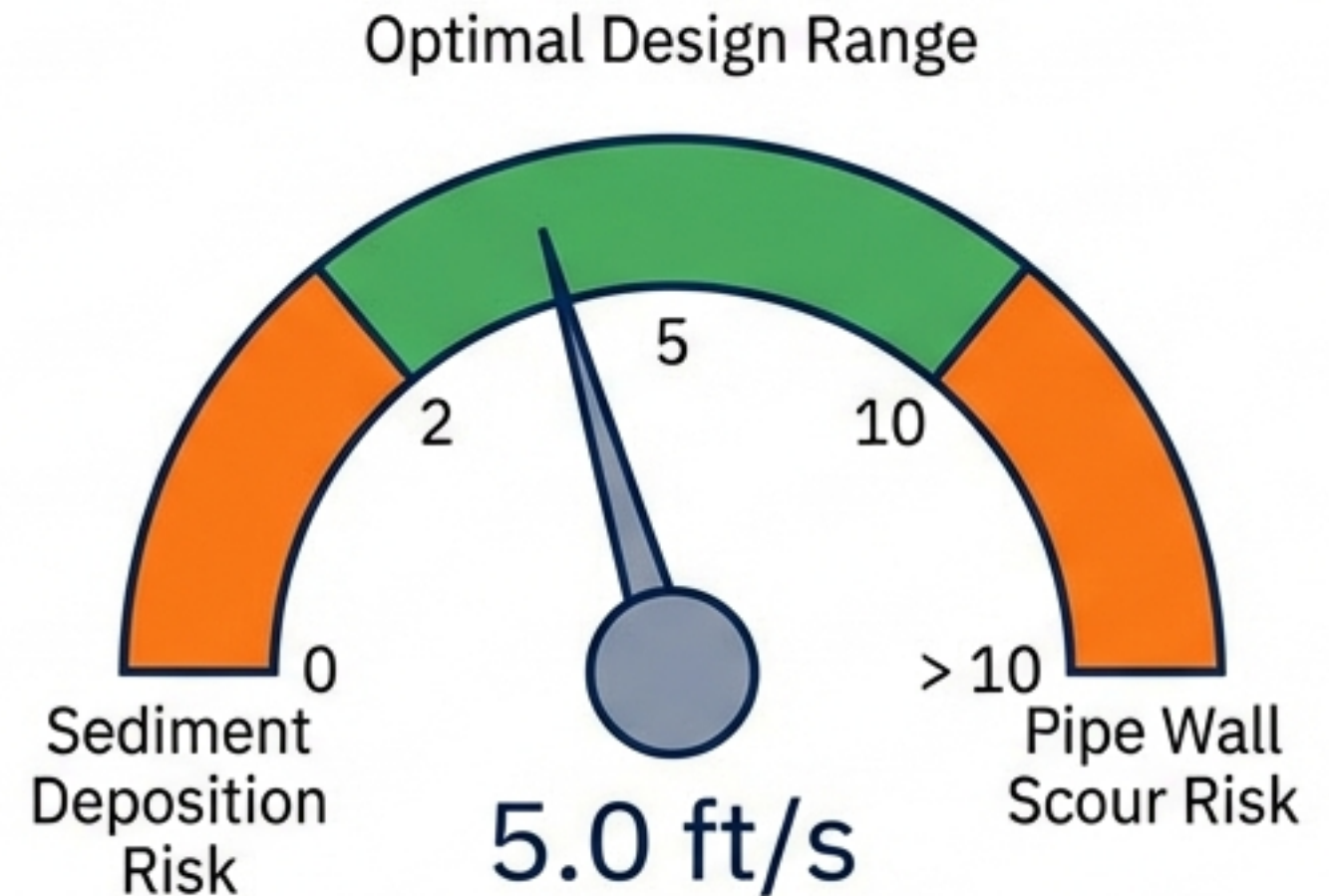
Velocity (V) decreases significantly as vegetation and roughness (n) increase.

# Application: Storm Sewer Design



Cross-Section: Circular Storm Sewer

## Velocity Design Considerations



Storm sewers act as gravity channels. Design must balance self-cleaning velocity against structural damage.

# Worked Example: Sizing a Storm Sewer

## Calculation Card



### Given Data:

- Required Flow (Q): 15 ft<sup>3</sup>/s
- Slope (S): 0.5%
- Material: Concrete (n = 0.013)

### Calculation:

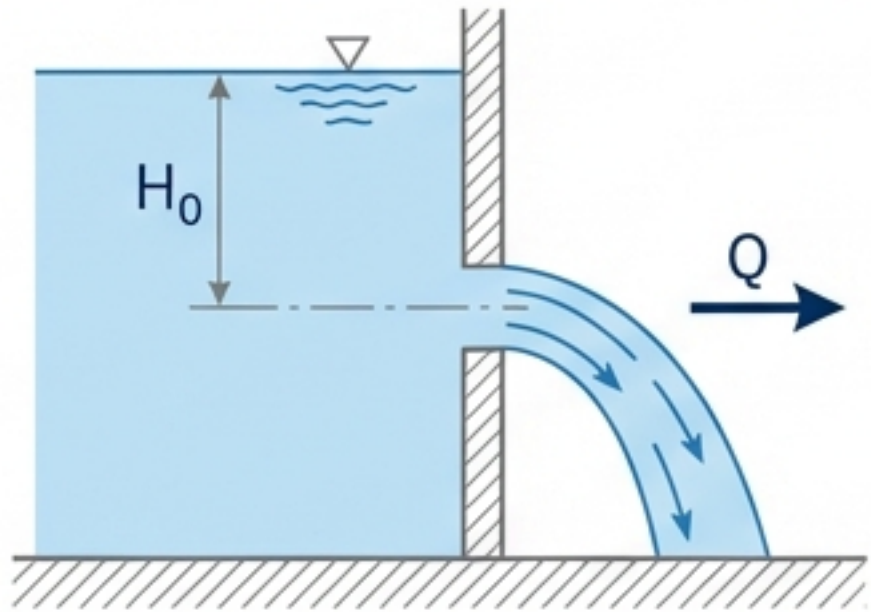
1. Assume full pipe flow.
2. Solve Manning's for Diameter (D).

### Result:

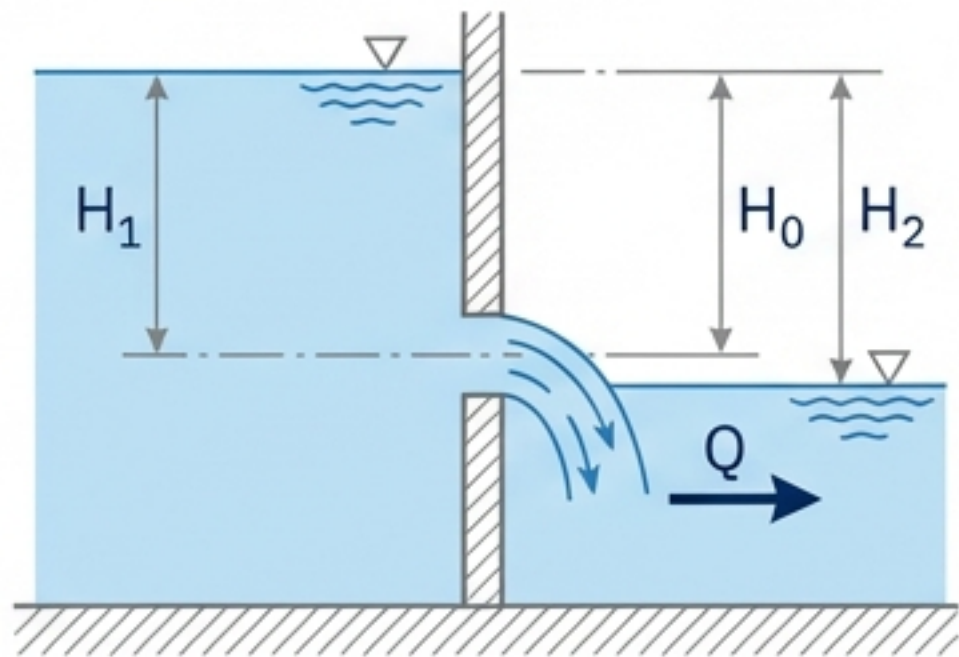
Required Diameter = 2.0 ft  
Verification Velocity = 4.78 ft/s  
(Acceptable)

# Phase 3: Outlet Control — Orifices

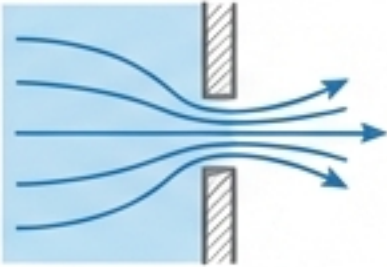
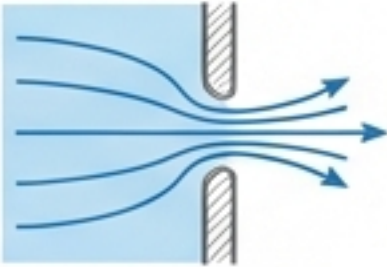
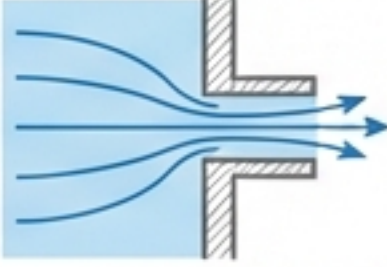
## UNSUBMERGED ORIFICE



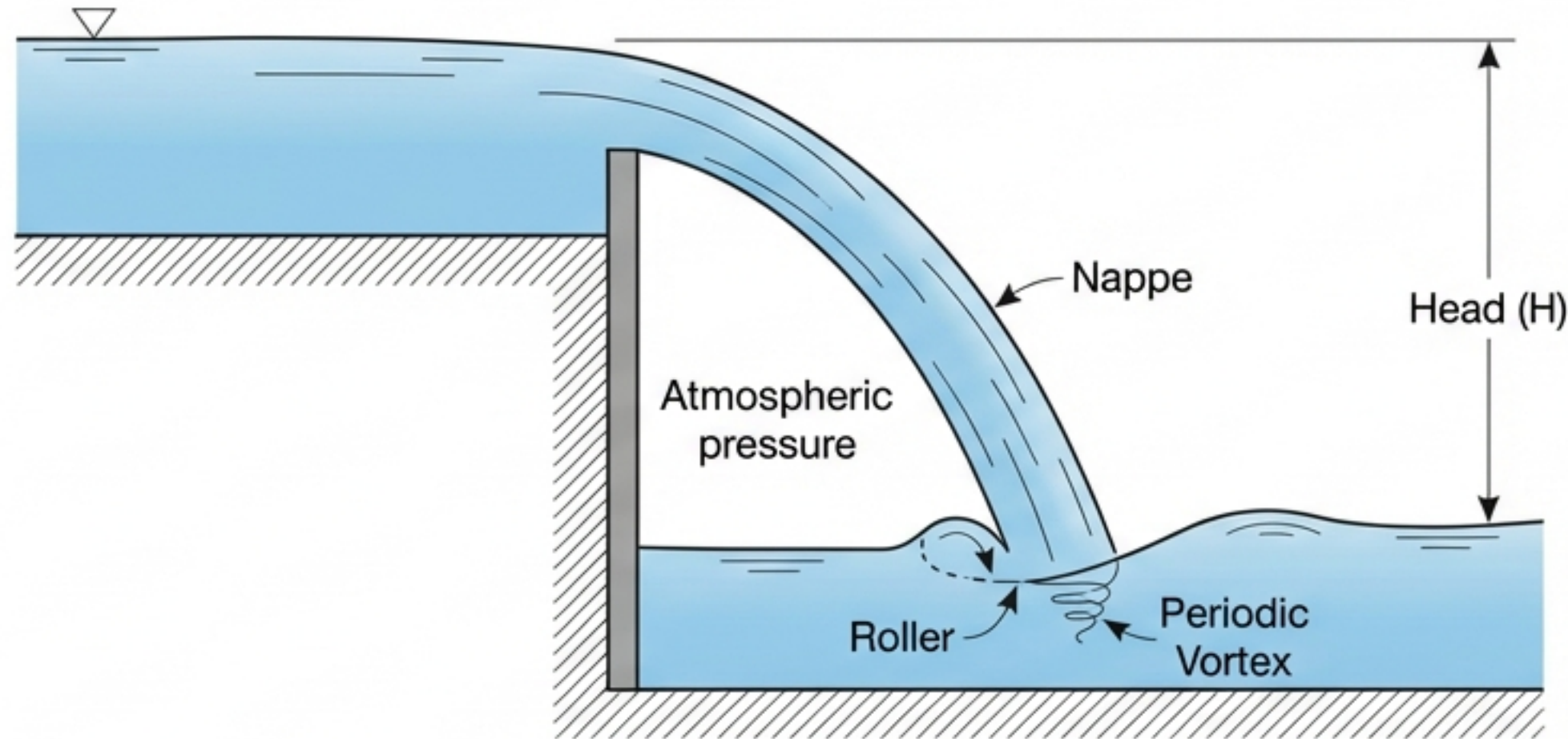
## SUBMERGED ORIFICE



Discharge Equation:  $Q = C_d \times A \times \sqrt{2gh}$

Discharge Coefficients ( $C_d$ )		
Illustration	Type	Coefficient (C)
	Sharp Edge	$C = 0.61$
	Rounded Edge	<b><math>C = 0.98</math></b> <b>(Most Efficient)</b>
	Short Tube	$C = 0.80$

# Outlet Control — Weirs



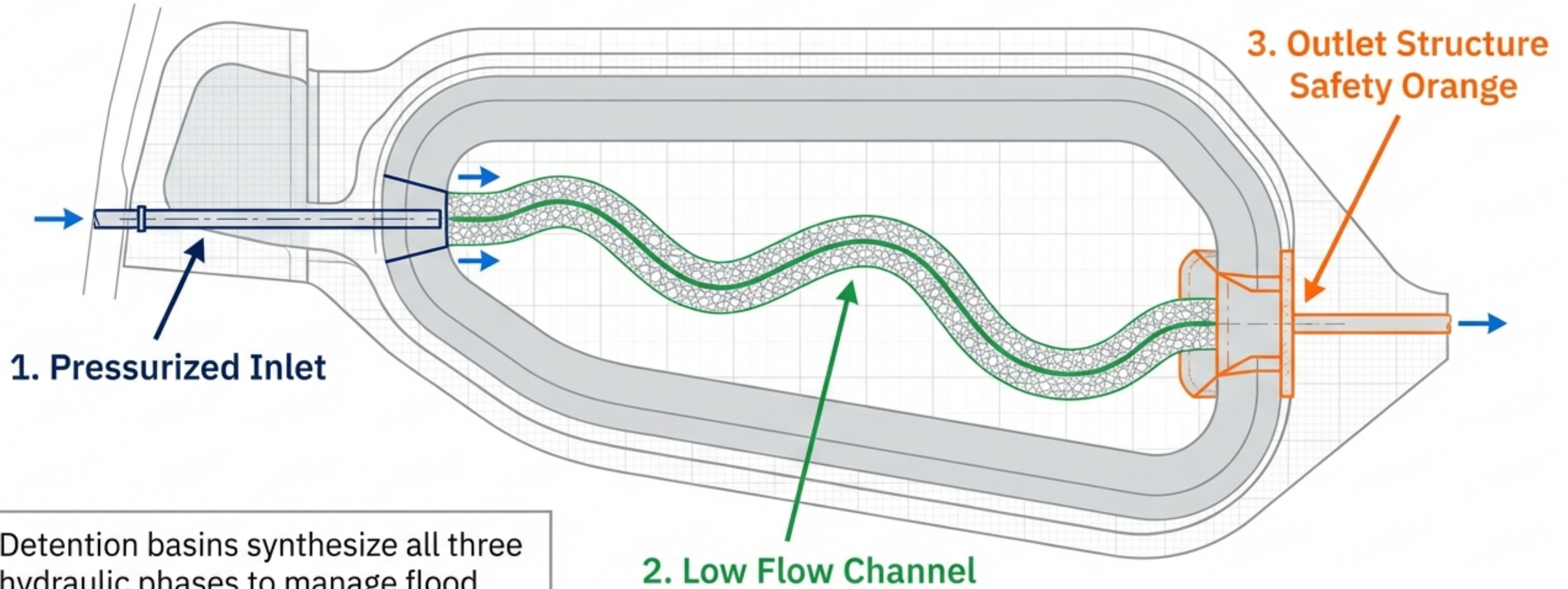
## The Rehbock Formula

The weir coefficient ( $C_w$ ) is not constant. It depends on the ratio of Head ( $H$ ) to Weir Height ( $P$ ).

Rectangular Weir Equation:

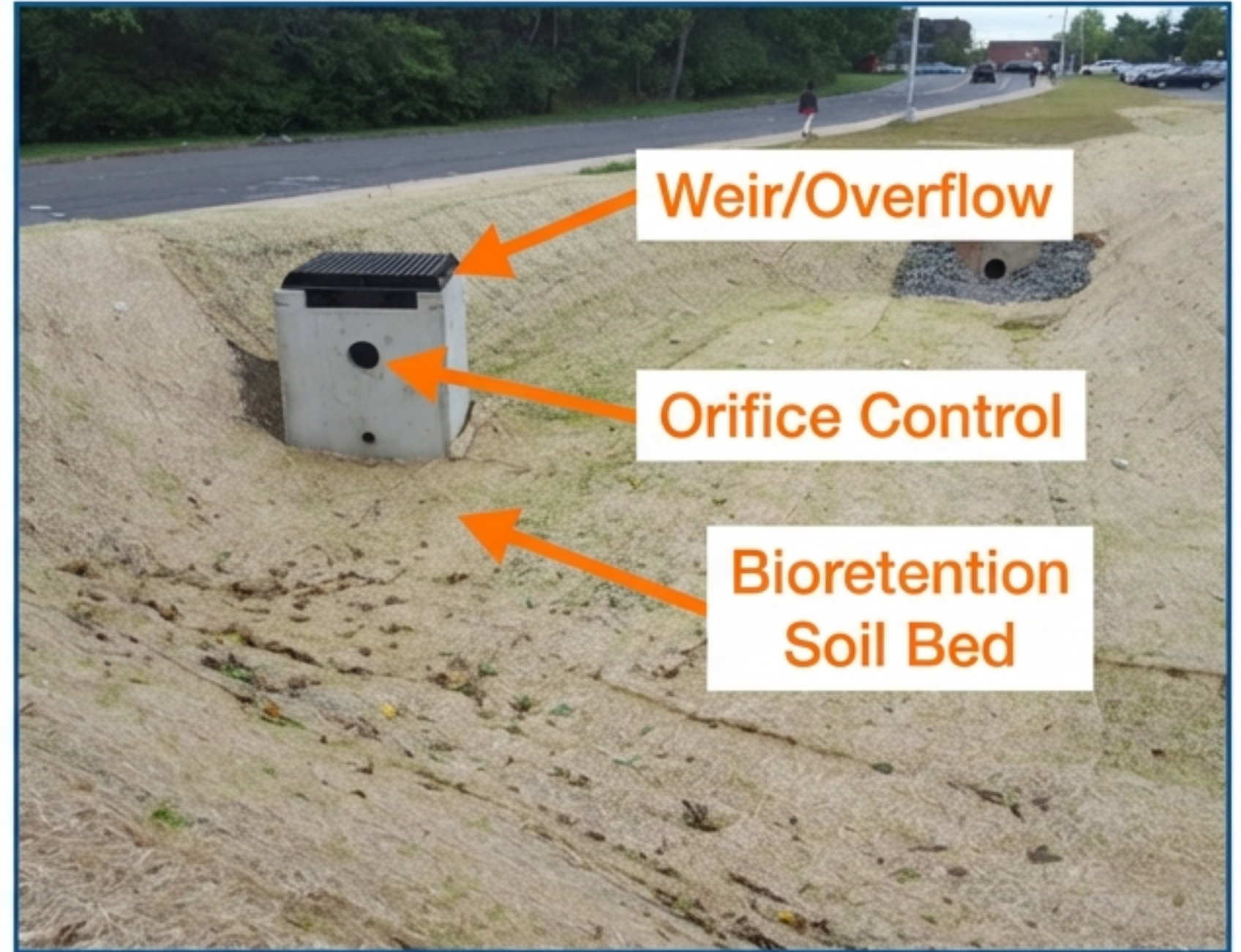
$$Q = C_w \times \frac{2}{3} \times \sqrt{2g} \times B \times H^{3/2}$$

# Integrated Systems: The Detention Basin



Detention basins synthesize all three hydraulic phases to manage flood peaks and water quality.

# Real-World Application: Rutgers Busch Campus



Bioretention system at Brett & Bartholomew Roads, constructed 2016.

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03	11/15/2023	E-301