

DYNAMICS OF ESTUARINE PROCESSES

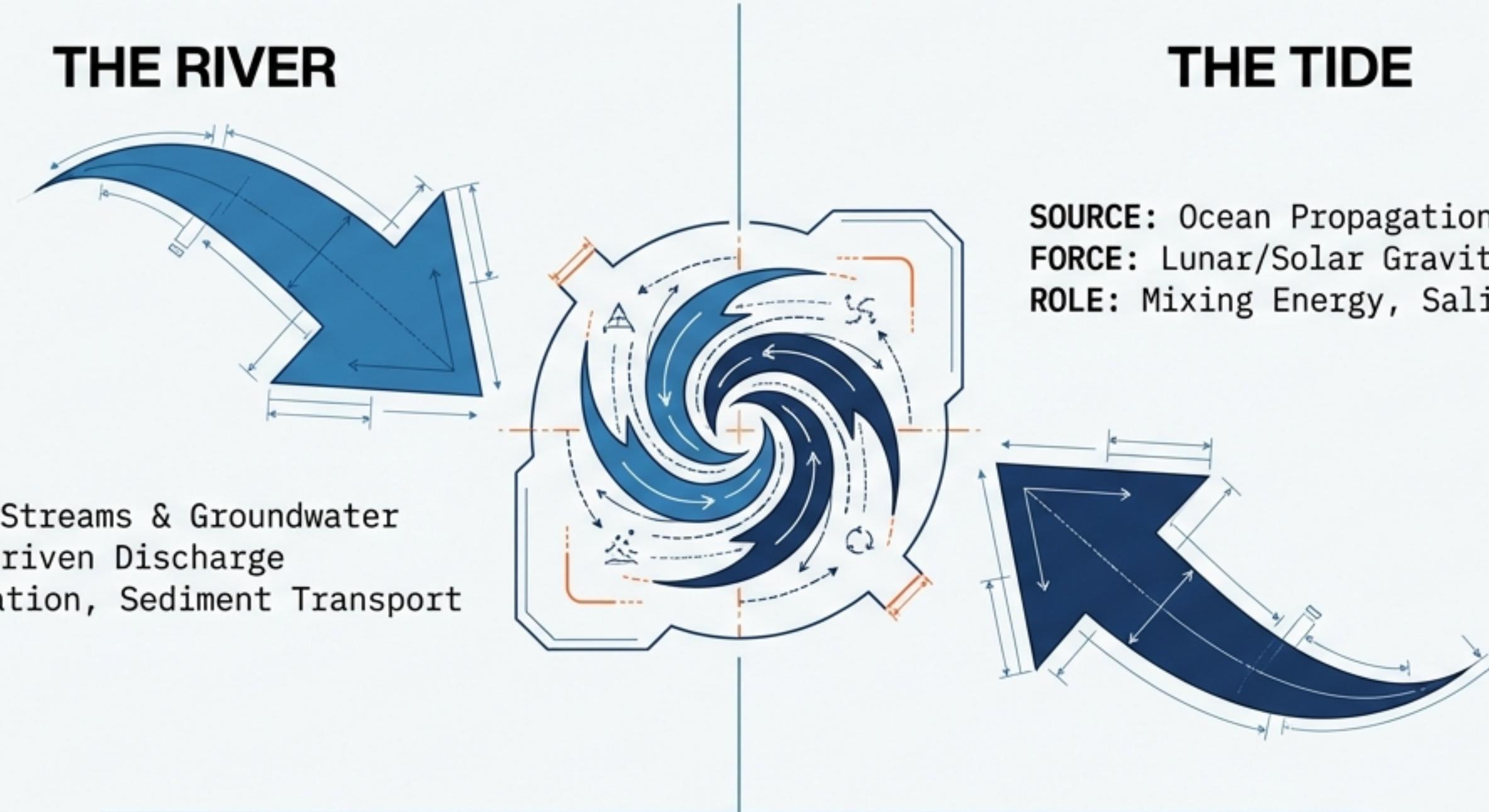
Freshwater Inflow • Tidal Propagation • Hydrodynamic Modeling

SCOPE: An engineering overview of the interaction between freshwater discharge and tidal saline flux.

THE ESTUARINE SYSTEM: A COMPETITION OF FLUXES

THE RIVER

SOURCE: Surface Streams & Groundwater
FORCE: Gravity-driven Discharge
ROLE: Stratification, Sediment Transport

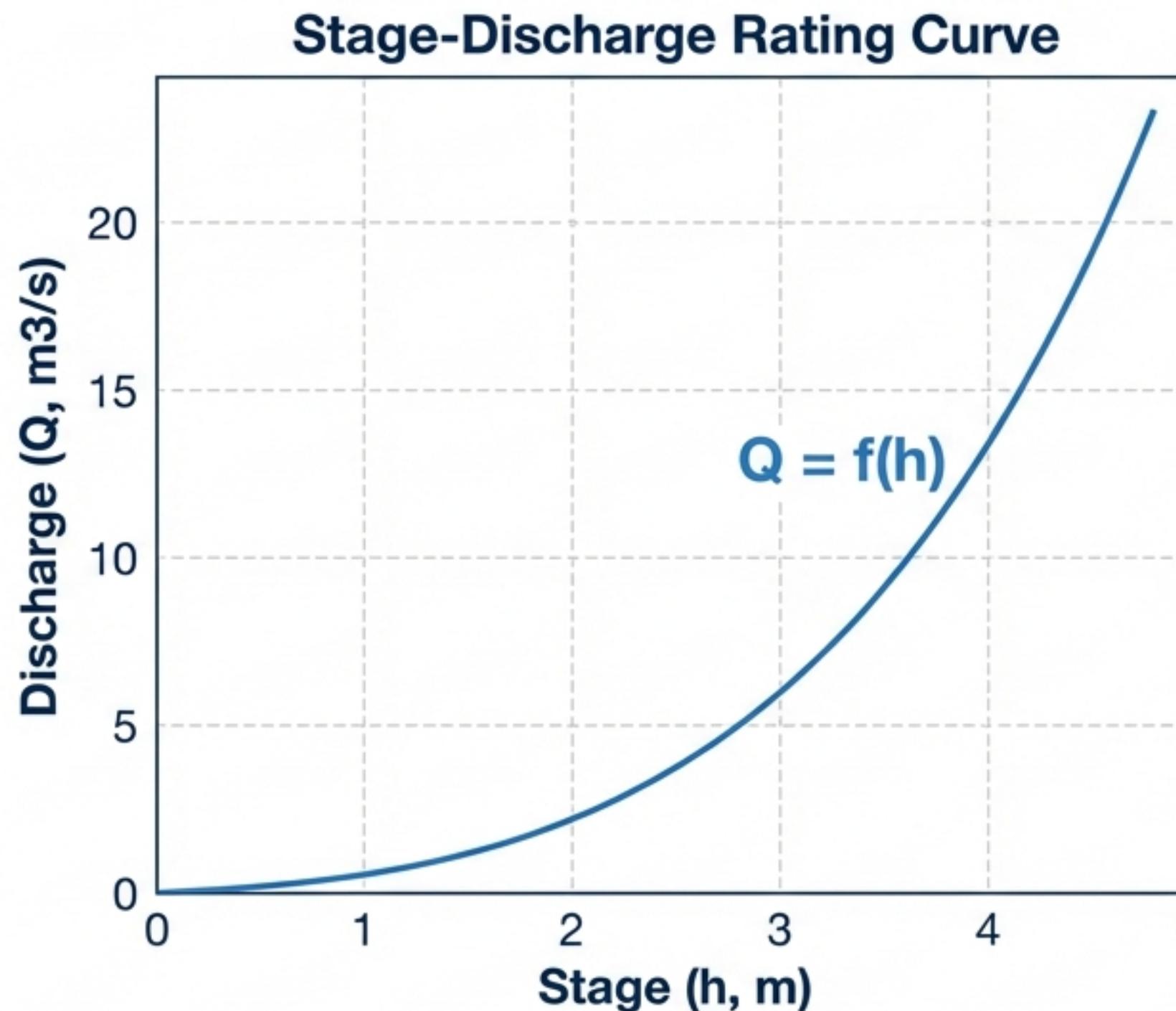


THE TIDE

SOURCE: Ocean Propagation
FORCE: Lunar/Solar Gravitational Potential
ROLE: Mixing Energy, Salinity Intrusion

KEY INSIGHT: The balance of these opposing forces determines the classification and biological health of the system.

INPUT I: QUANTIFYING SURFACE STREAMFLOW



Manning's Equation

$$Q = \frac{1}{n} \cdot A \cdot R^{2/3} \cdot S^{1/2}$$

Roughness
Coefficient
(Friction)
IBM Plex Mono

n Values

$n=0.015$, Concrete

Slate Terracotta

$n=0.050$, Dense Weeds

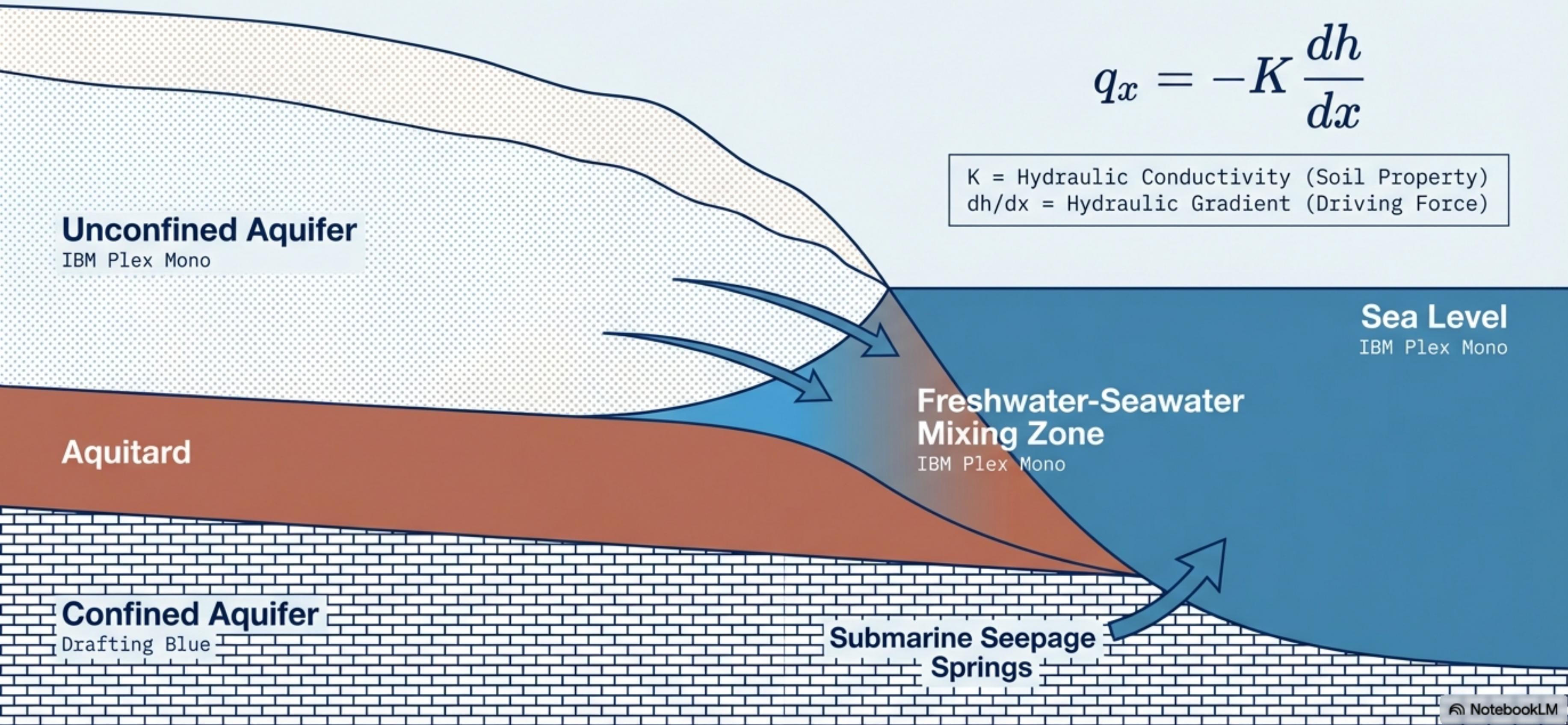
Hydraulic
Radius
(Efficiency)

Slope
(Energy
Grade Line)

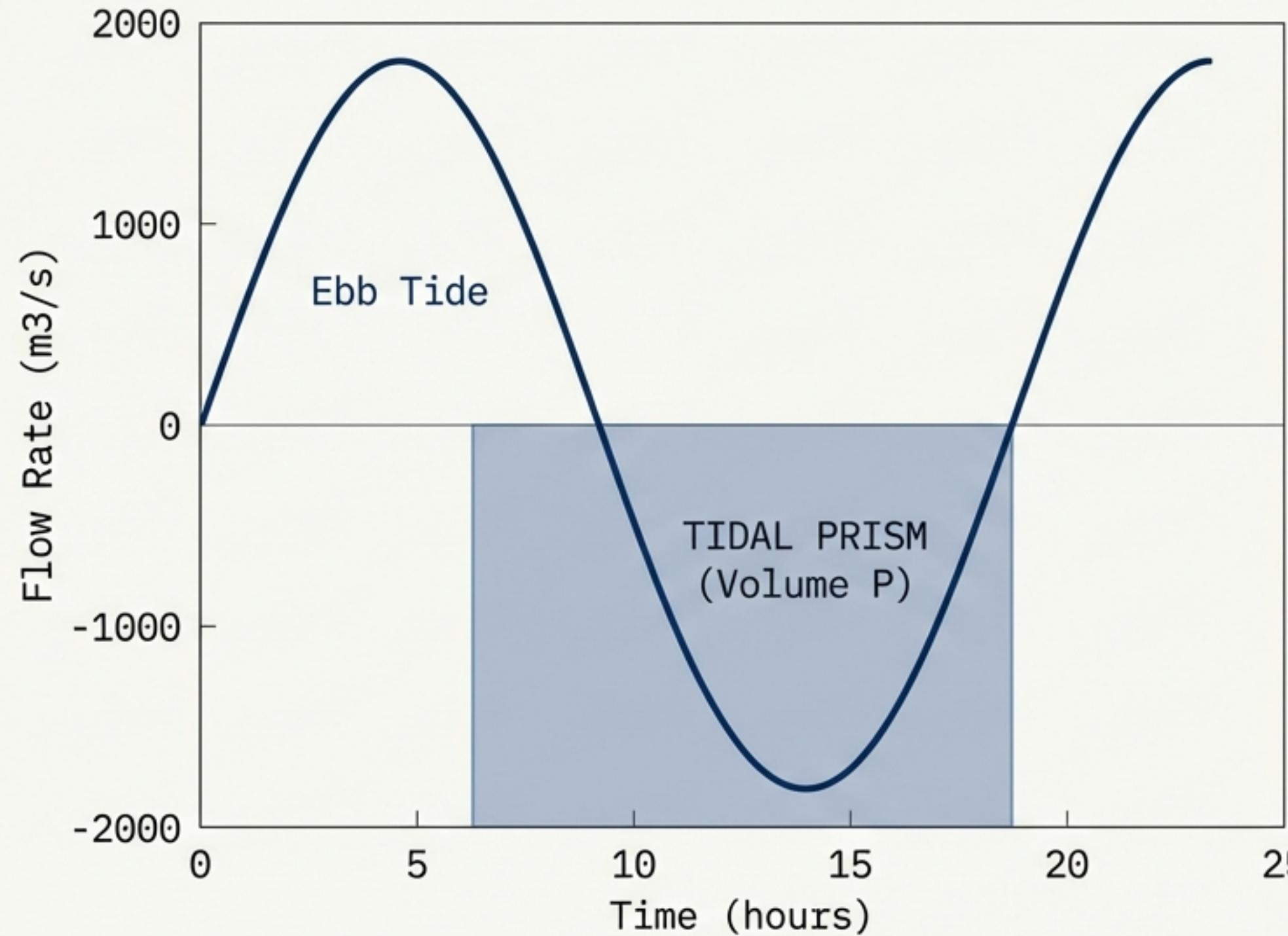
INPUT II: THE INVISIBLE FLOW OF GROUNDWATER

$$q_x = -K \frac{dh}{dx}$$

K = Hydraulic Conductivity (Soil Property)
 dh/dx = Hydraulic Gradient (Driving Force)



THE TIDAL PRISM: MEASURING THE ESTUARY'S INHALE



DEFINITION: Total volume entering during flood tide.

APPROXIMATION FORMULA:

$$P = (2/\pi) * A_f * T_f$$

WHERE:

A_f = Peak Flood Flow

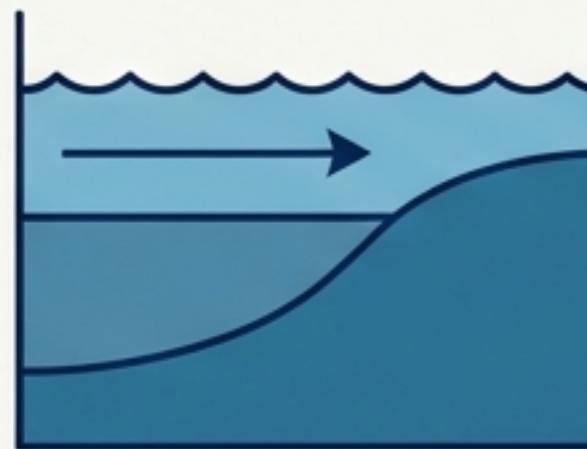
T_f = Flood Duration

EXAMPLE:

If Peak = 1800 m^3/s and Duration = 6.21 hrs:

Prism Volume \approx 25.6 Million m^3

CLASSIFICATION: THE SIMONS RATIO

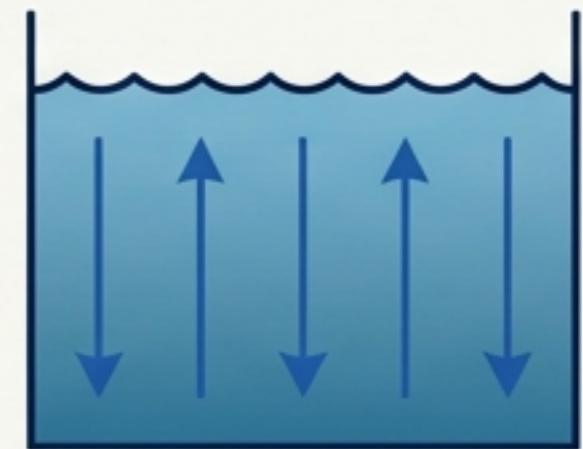


RATIO > 1.0

Slate Terracotta

STRATIFIED.
River Dominant.
Salt wedge structure.

$$\text{Simons Ratio} = \frac{(\text{Vol. Freshwater Inflow})}{(\text{Tidal Prism})}$$

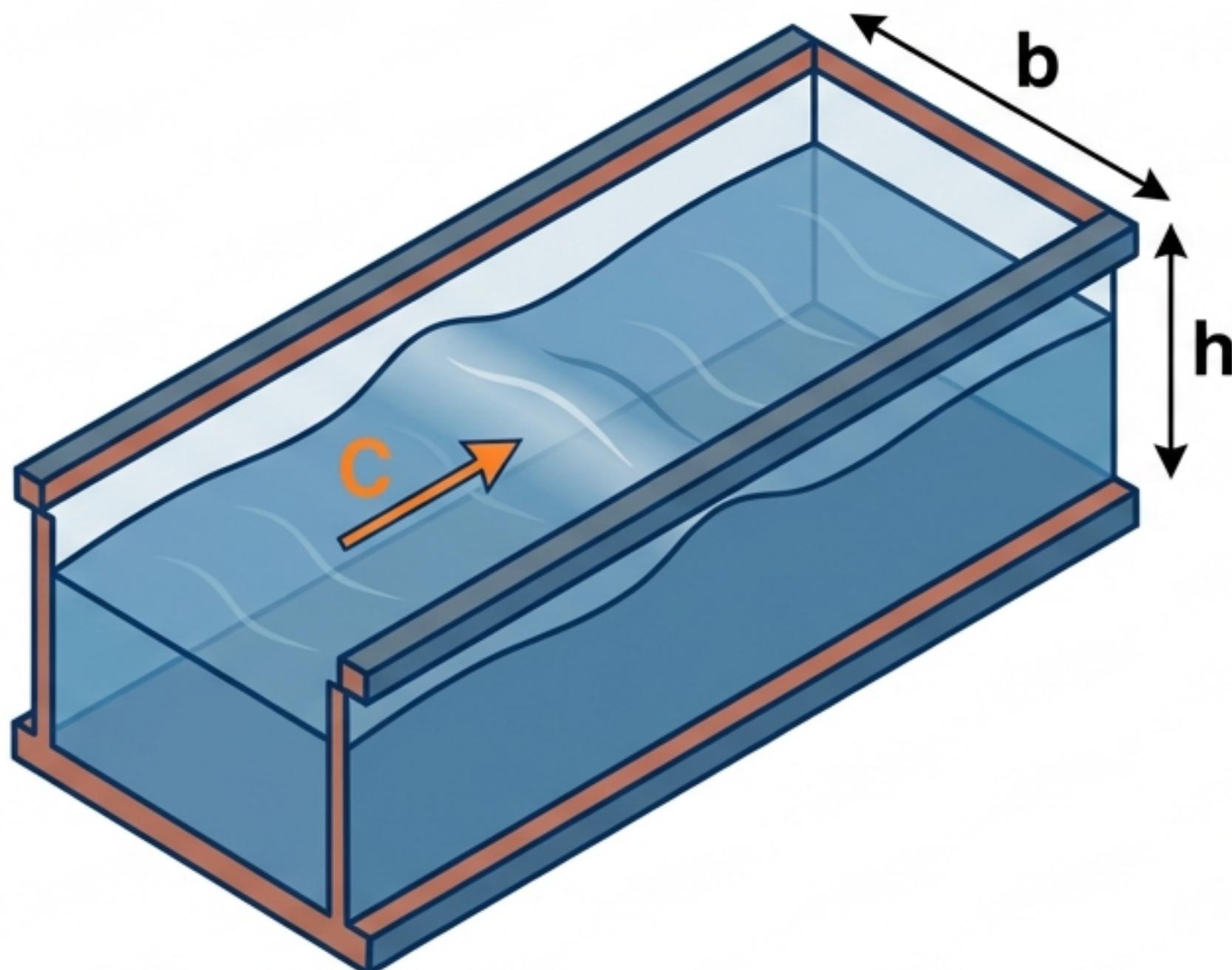


RATIO < 0.1

Nautical Navy

WELL-MIXED.
Tide Dominant.
Vertical homogeneity.

FUNDAMENTALS OF TIDAL DYNAMICS



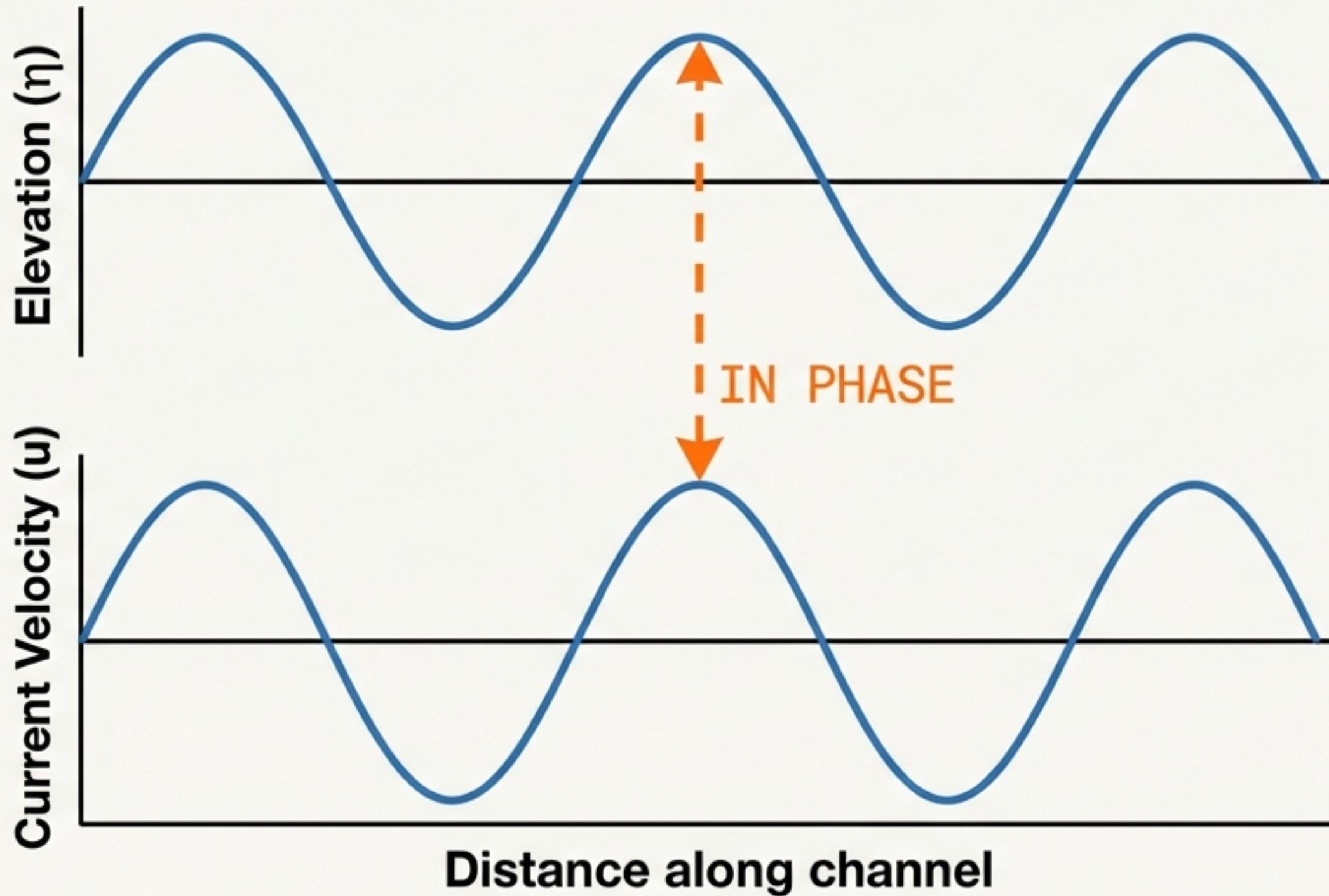
Celerity Equation

$$C_0 = \sqrt{gh}$$

- Idealized Assumptions
 - Frictionless Channel
 - Rectangular Cross-Section
 - Small Amplitude relative to Depth
 - Wave speed depends ONLY on water depth

CASE A: INFINITE LENGTH, NO FRICTION

The Progressive Wave



BEHAVIOR:

The wave travels indefinitely without reflection.

Maximum High Water occurs at Maximum Flood Velocity.

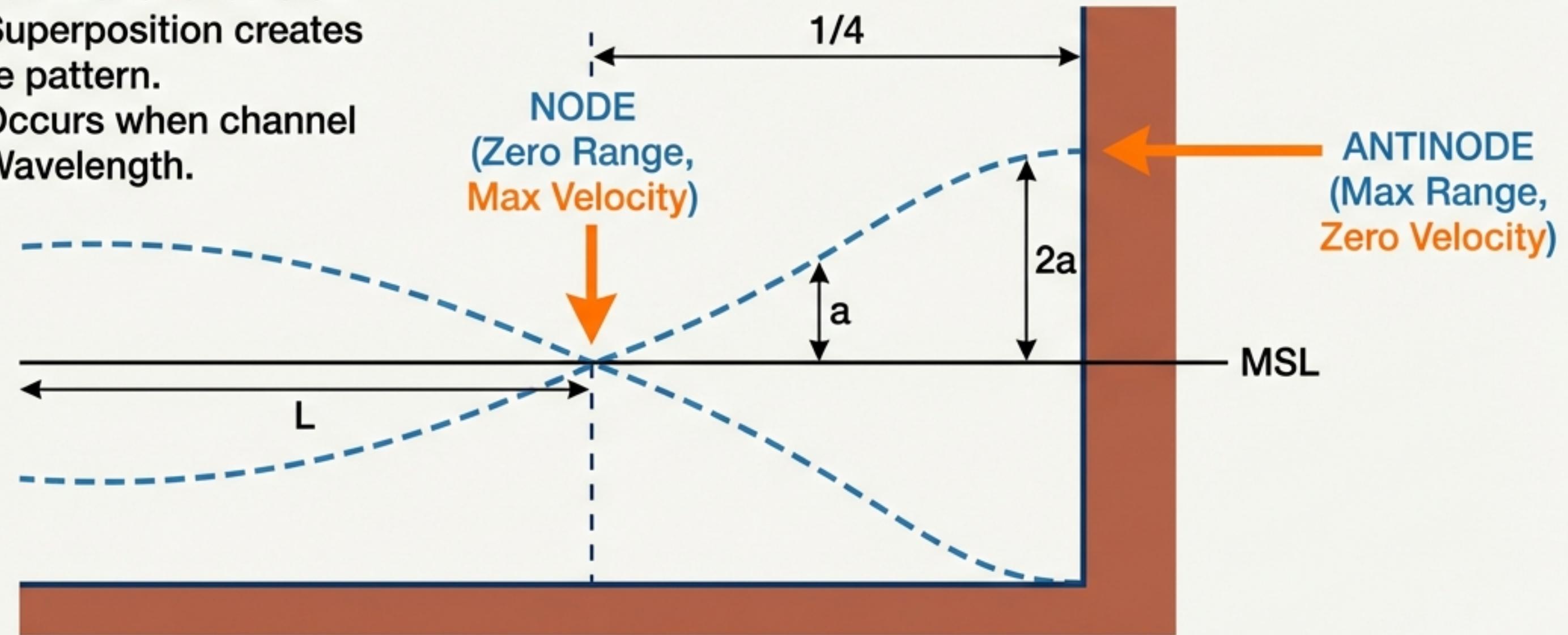
Maximum Low Water occurs at Maximum Ebb Velocity.

CASE B: CLOSED END, NO FRICTION

The Standing Wave (Reflection)

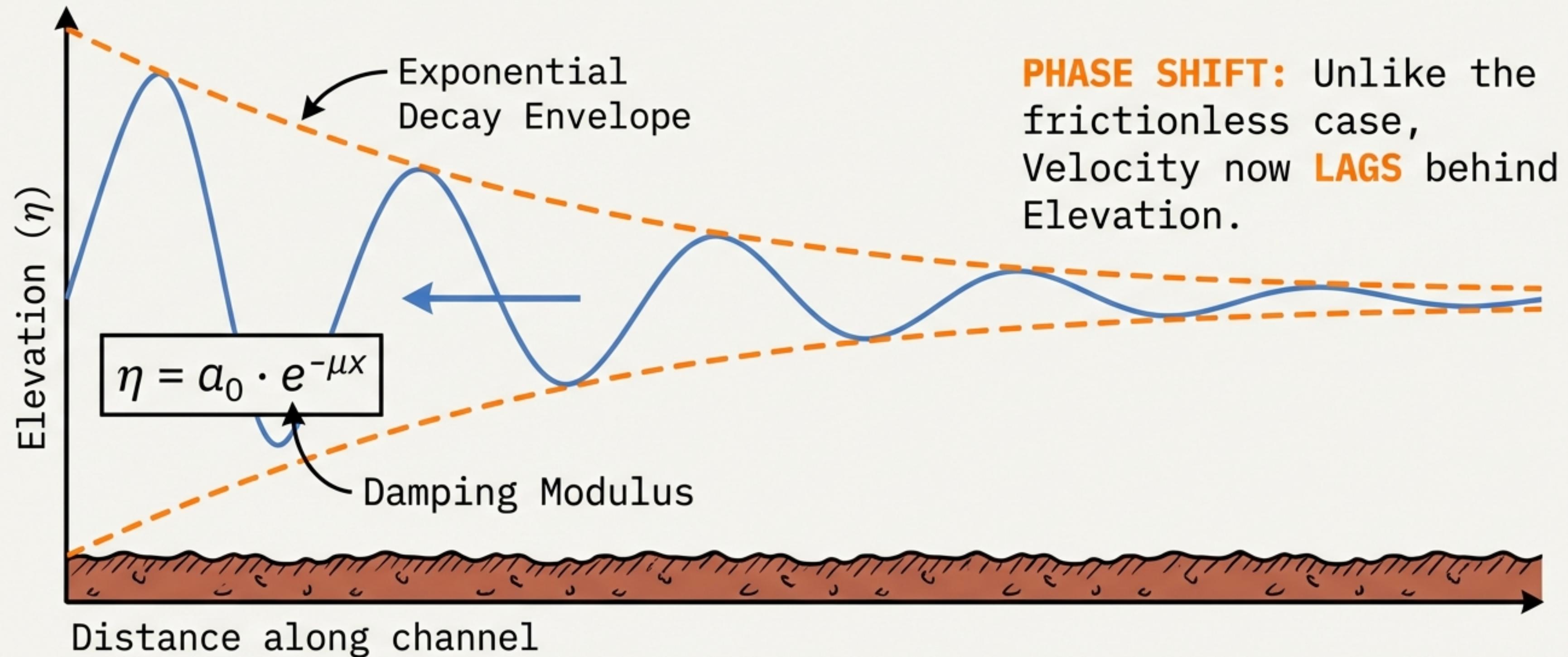
PHYSICS: Incident wave reflects off the boundary. Superposition creates a standing wave pattern.

RESONANCE: Occurs when channel length $L = 1/4$ Wavelength.



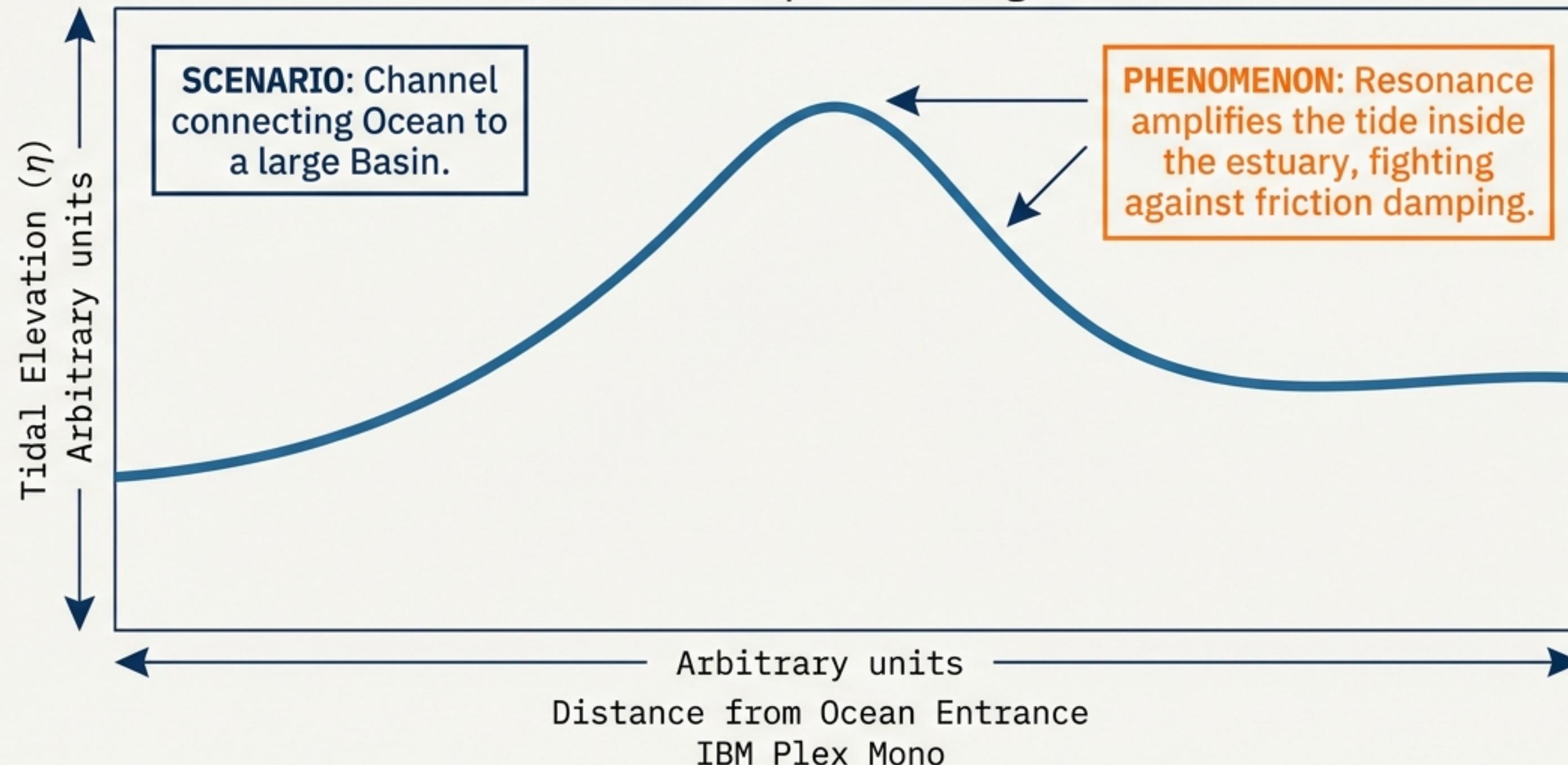
CASE C: INTRODUCING FRICTION (THE REALITY)

Damped Progressive Wave



REAL-WORLD COMPLEXITY: CO-OSCILLATING TIDES

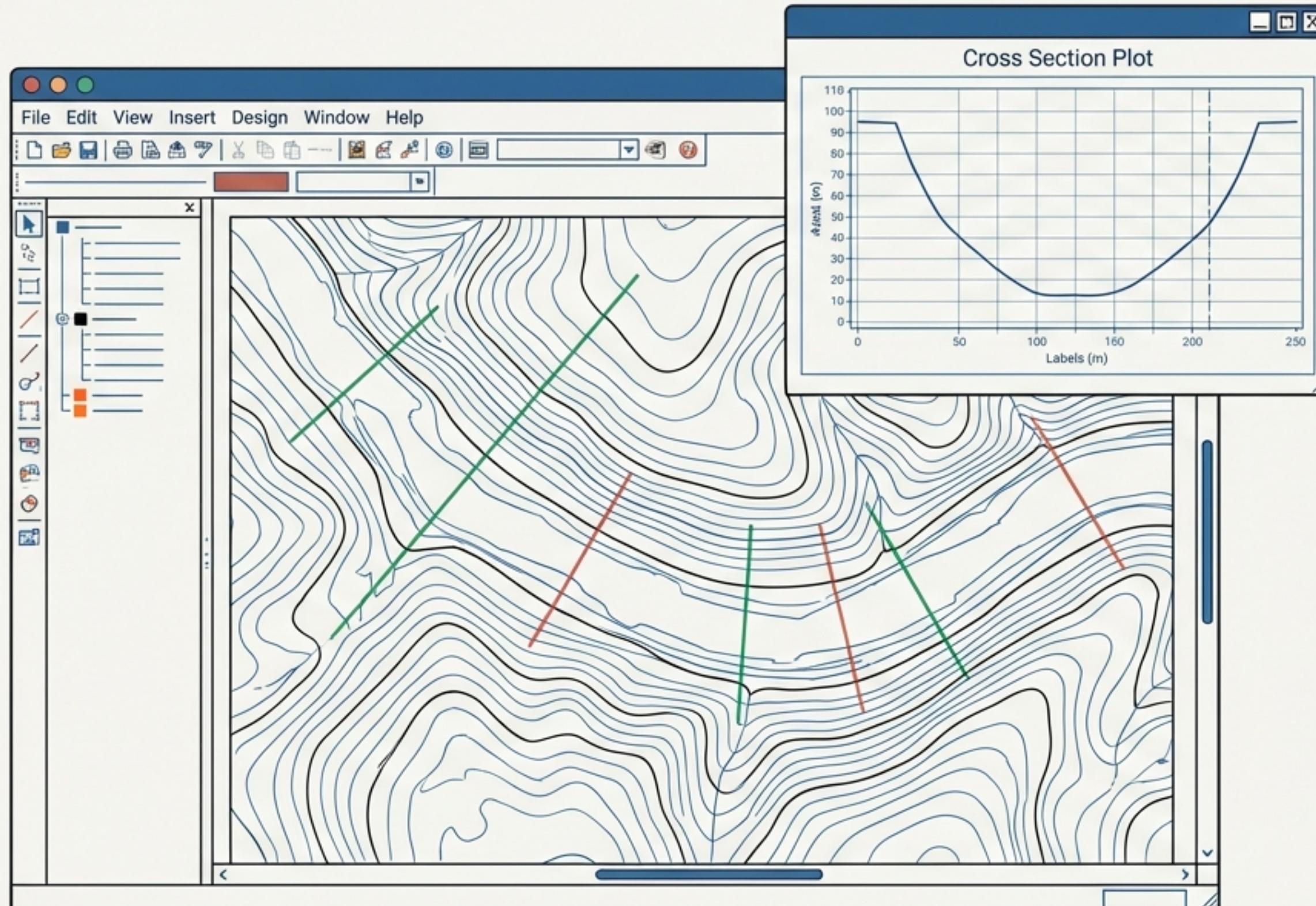
Variation of Amplitude along Channel.



OBSERVED TIDAL PASSAGE VELOCITIES

LOCATION	PEAK VELOCITY (m/s)	TIDAL RANGE	NOTES
Cook Inlet, AK	4.0 m/s (Flood)	10-11 m	Extreme bore & range
Deception Pass, WA	5.1 m/s	2.6 m	High-energy jet
Lubec Narrows, ME	3.09 m/s	5.6 m	Ultra-constricted
East River, NY	3.0 m/s	2.3 m	Swift, moderate range

NUMERICAL MODELING WITH HEC-RAS



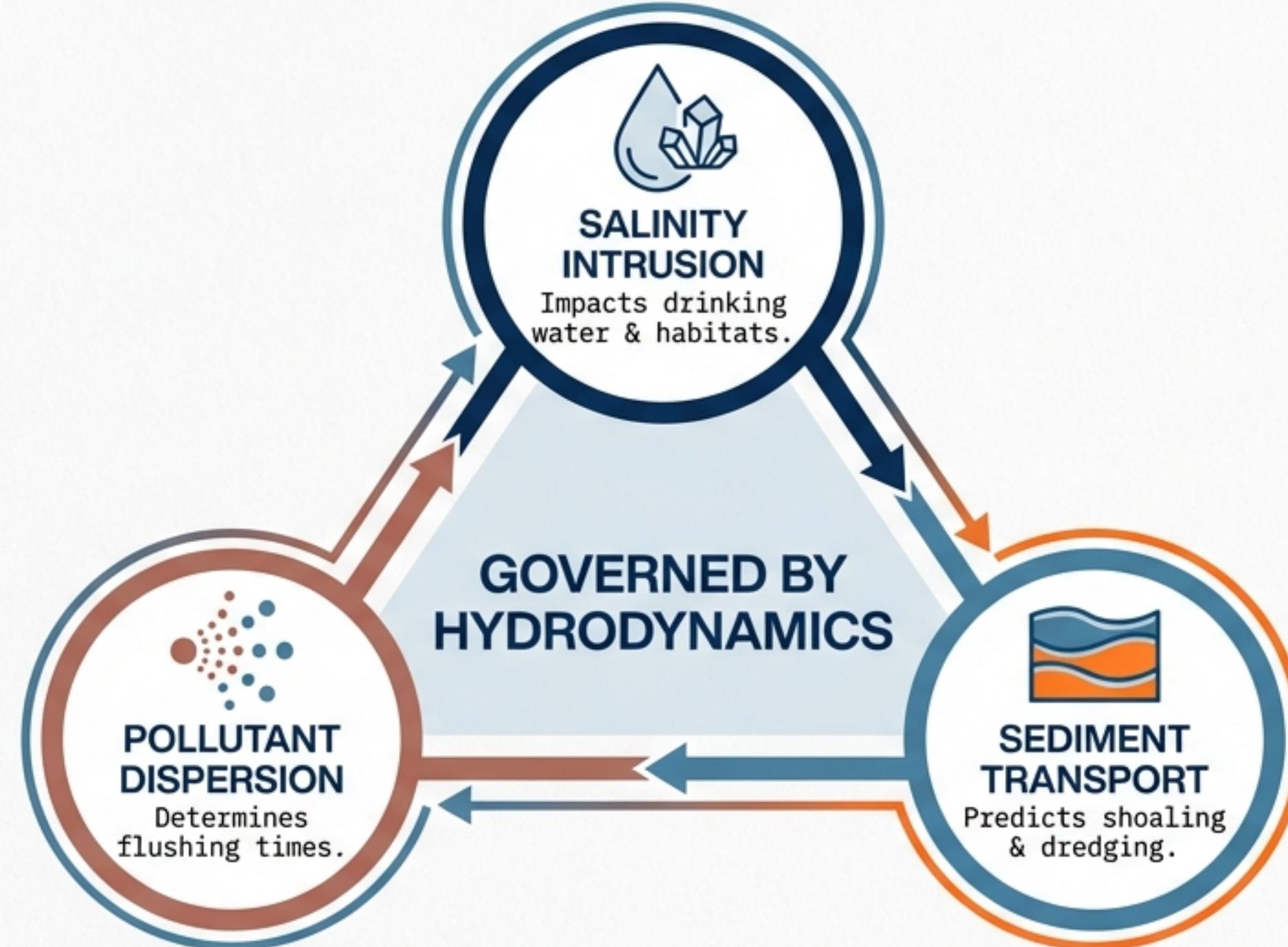
TOOL: HEC-RAS (River Analysis System)

- 1D Steady & Unsteady Flow
- Sediment Transport
- Water Temperature Modeling
- Handling Irregular (Non-Rectangular) Geometries

SUMMARY OF TIDAL PROPAGATION REGIMES

REGIME	WAVE TYPE	PHASE RELATION	AMPLITUDE BEHAVIOR
Infinite / Frictionless	Progressive Wave	In Phase (0°)	Constant (No Decay)
Closed End / Frictionless	Standing Wave (Reflected)	90° Shift (Nodes/Antinodes)	Doubles at Wall (Resonance)
Real Estuary (Friction)	Damped Co-oscillating	Velocity Lags Elevation	Exponential Decay ($e^{-\mu x}$)

IMPLICATIONS FOR ESTUARINE MANAGEMENT



CONCLUSION: The estuary is a dynamic integration of hydrological inputs and tidal physics. Accurate modeling is the foundation of sustainable coastal engineering.